DRAINAGE REPORT

Slate Upper School | 5100 Ridge Road | North Haven, Connecticut

October 27, 2020
MMI #6156-03-05

This Drainage Report has been prepared in support of the proposed Slate Upper School project to be located at 5100 Ridge Road in the town of North Haven, Connecticut. The site is currently occupied by the existing Mount Carmel Christian Church building. The project proposes to renovate the existing building and parking area and construct a new private school that will have one new building, a central plaza area, new surface parking spaces, a new bituminous driveway off Ridge Road, and all the associated site infrastructure necessary to support the proposed use.

Figure 1 – #5100 Parcel

TABLE 1
Stormwater Data

Site Total Area	2.97 acres
Existing On-Site Impervious Area	0.40 acres
Proposed On-Site Impervious Area	1.08 acres
Soil Types (Hydrologic Soil Group)	"B," "C," and "D"
Existing Land Use	Woods, open space, gravel, building, parking lot, and bituminous road
Proposed Land Use	Woods, open space, building, parking lot, sidewalk, pavers, and bituminous road
Design Storm for Stormwater Management	No increases in peak rates of runoff for the 2-, 10-, 25-, 50-, and 100-year storms. First-flush runoff retention (CTDEEP WQV).
Water Quality Measures	2-foot-sump catch basins, hydrodynamic separator, riprap energy dissipator, sediment forebay, retention storage, and riprap level spreader
Design Storm for Storm Drainage	25-year storm
Federal Emergency Management Agency Special Flood Hazard Areas	Zone X (Area of Minimal Flood Hazard)
Connecticut Department of Energy & Environmental Protection Aquifer Protection Areas	Mount Carmel - Level A

STORMWATER MANAGEMENT APPROACH

The stormwater management system for this site has been designed utilizing Best Management Practices (BMPs) to provide water quality management while attenuating the proposed peak-flow rates from the new development. The design goal is to provide water quality treatment in accordance with the Connecticut Department of Energy & Environmental Protection (CTDEEP) requirements for Water Quality Volume (WQV) and prevent increases in the predevelopment runoff rates from the project site. Existing drainage patterns will be maintained to the maximum extent practicable, and a stormwater treatment train is proposed, including several water quality measures such as catch basins with 2-foot sumps, a hydrodynamic separator, a riprap energy dissipator, a sediment forebay, a level spreader, and retention volume within the proposed stormwater management basin.

The proposed project will include one stormwater management basin that is designed to detain the proposed stormwater peak discharge rates and provide retention storage to address water quality. The proposed detention basin is designated on the site plans as Stormwater Basin 110 and will have an outlet control structure made of reinforced concrete. The stormwater runoff discharge from the basin will be conveyed to a riprap level spreader, which will overflow toward the existing wetland system to the west.

The computer program entitled *Hydraflow Storm Sewers Extension for AutoCAD® Civil 3D® 2019* by Autodesk, Inc., Version 10.5, was used for designing the proposed storm drainage collection system. Storm drainage computations performed include pipe capacity and hydraulic grade line calculations. The contributing watershed to each individual catch basin inlet was delineated to determine drainage area and land coverage. These values were used to determine the stormwater runoff to each inlet using the Rational Method. The rainfall intensities for the site were obtained from the National Oceanic and Atmospheric Administration (NOAA) Atlas 14, Volume 10, Precipitation Frequency Data Server (PFDS). The proposed storm drainage system is designed to provide adequate capacity to convey the 25-year storm event.

WATER QUALITY MANAGEMENT

Stormwater runoff from the proposed improvements will be collected by a subsurface pipe and catch basin drainage system. The proposed drainage system will include catch basins with 2-foot sumps that will trap sediments.

A hydrodynamic separator such as the CDS system, manufactured by Contech Engineered Solutions, will be installed in the storm drainage system prior to discharging into the proposed stormwater basin. This unit will further remove suspended solids before discharging downgradient, which will in turn remove other pollutants that tend to attach to the suspended solids and effectively remove other debris and floatables that may be present in stormwater runoff. The CDS unit has been designed to meet criteria recommended by the CTDEEP *2004 Stormwater Quality Manual*. The device was designed based on the determined Water Quality Flow (WQF), which is the peak flow rate associated with the WQV, and sized based on the manufacturer's specifications.

A sediment forebay is proposed around the proposed drainage pipe outlet locations into the stormwater management basin, which will improve water quality by trapping floatables as well as filtering coarse sediment and other pollutants. The forebay will be constructed using a riprap filter berm and riprap splash pads. The proposed riprap splash pads will dissipate the potential erosive velocity of stormwater entering the basin as well as trap sediments. The sediment forebay will contain the deposited sediment within a small area of the basin and will allow for maintenance accessibility.

The stormwater basin will provide retention volume along its bottom thus creating a water quality feature within it. This serves several purposes, including stormwater renovation and first-flush retention. The vegetation will provide pollutant removal by filtering stormwater runoff and utilizing excess nutrients that may be present in the stormwater. The CTDEEP *2004 Stormwater Quality Manual* (Chapter 7) recommends methods for sizing stormwater treatment measures with WQV computations. The WQV addresses the initial stormwater runoff, also commonly referred to as the "first flush" runoff. The WQV provides adequate volume to store the runoff associated with the first 1 inch of rainfall, which tends to contain the highest concentration of potential pollutants. Supporting calculations have been included in the Appendix of this report.

The riprap level spreader system was designed to safely release the stormwater discharge from the stormwater basin. The design calls for a level stone berm as an overflow outlet, which will be set against a precast concrete curb. The stone level spreader will gradually release stormwater in a quiescent manner as sheet flow rather than a concentrated point discharge that results from typical storm pipe outlets or flared end sections.

HYDROLOGIC ANALYSIS

A hydrologic analysis was conducted to analyze the predevelopment and postdevelopment peak-flow rates from the site. The ultimate stormwater runoff discharge from the site is toward the wetland system that abuts the northern and western property boundaries, which was chosen as the analysis point for this hydrologic analysis. The upstream areas that drain onto the site were also incorporated as part of the analysis. The total combined watershed area delineated is approximately 4.8 acres under both existing and proposed conditions.

The method of predicting the surface water runoff rates utilized in this analysis was a computer program entitled *Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2019* by Autodesk, Inc., Version 2020. The *Hydrographs* program is a computer model that utilizes the methodologies set forth in the *Technical Release No. 55* (TR-55) manual and *Technical Release No. 20* (TR-20) computer model, originally developed by the United States Department of Agriculture – Natural Resources Conservation Service (USDA-NRCS). The *Hydrographs* computer modeling program is primarily used for conducting hydrology studies such as this one.

The *Hydrographs* computer program forecasts the rate of surface water runoff based upon several factors. The input data includes information on land use, hydrologic soil type, vegetation, contributing watershed area, time of concentration, rainfall data, storage volumes, and the hydraulic capacity of structures. The computer model predicts the amount of runoff as a function of time, with the ability to include the attenuation effect due to dams, lakes, large wetlands, floodplains, and stormwater management basins. The input data for rainfalls with statistical recurrence frequencies of 2, 10, 25, 50, and 100 years was obtained from the NOAA Atlas 14, Volume 10, database. The corresponding rainfall totals are listed below.

Storm Frequency	**Rainfall (inches)**
2 year	3.50
10 year	5.44
25 year	6.65
50 year	7.54
100 year	8.51

Land use for the site under existing and proposed conditions was determined from field survey, town topographic maps, and aerial photogrammetry. Land use types utilized in the analysis included woods, grassed or open space, gravel, building, and impervious (paved) cover. Soil types in the watershed were determined from the CTDEEP Geographic Information System (GIS) database of the USDA-NRCS soil survey for New Haven County, Connecticut. The different land uses and soil types were utilized to determine composite runoff Curve Numbers (CN) for each subwatershed. The time of concentration (Tc) was estimated for each subwatershed using the TR-55 methodology, which was computed by summing all travel times through the watershed as sheet flow, shallow concentrated flow, and channel flow.

The existing conditions were modeled with the *Hydrographs* program to determine the peak-flow rates for the various storm events at the analysis point. A revised model was developed incorporating the proposed site conditions and the stormwater management basin. The stormwater flows obtained with the revised model were then compared to the results from the existing conditions model. Peak-flow rates from the project site were controlled by the storage volume provided within the proposed stormwater basin and the hydraulic capacity of the outlet control structure. The basin has been designed to provide a minimum of 1 foot of freeboard to the top of the proposed berm during the 100-year storm event. The following peak rates of runoff were obtained from the *Hydrographs* hydrology results:

Analysis Point A – Wetland System					
	Peak Runoff Rate (cubic feet per second)				
Storm Frequency (years)	**2**	**10**	**25**	**50**	**100**
Existing Conditions	4.6	11.1	15.6	18.9	22.7
Proposed Conditions	4.0	10.4	14.7	18.2	22.7

Detention Basin 110*					
	Water Surface Elevation (feet)				
Storm Frequency (years)	**2**	**10**	**25**	**50**	**100**
Proposed Conditions	156.9	157.43	157.67	157.82	157.92

***Top Elevation of Basin = 159.0 feet**

CONCLUSION

The results of the hydrologic analysis demonstrate that there will be no increases in peak-flow rates from the proposed development. This was achieved for the storm events modeled through a planned stormwater management system with detention provided in the proposed stormwater basin. The proposed development will also introduce a new stormwater treatment train consisting of several water quality measures such as catch basins with 2-foot sumps, a hydrodynamic separator, a riprap energy dissipator, and a sediment forebay and retention volume within the proposed stormwater management basin.

The hydrodynamic separator device will be employed to pretreat stormwater runoff generated from the proposed impervious surfaces prior to entering the receiving stormwater basin. A CDS unit, manufactured by Contech Engineered Solutions, was selected and sized based on the contributing WQF, which is the peak-flow rate associated with the WQV. Furthermore, the CTDEEP WQV has been provided within the retention storage area along the bottom of the proposed stormwater management basin. The stormwater runoff discharge from the stormwater basin will be directed to a riprap level spreader that will gradually release stormwater runoff to the wetland system to the west.

All supporting documentation and stormwater-related computations are attached to this report along with the *Hydraflow Hydrographs* model results for stormwater management and *Hydraflow Storm Sewers* model results for the proposed storm drainage system. Illustrative watershed maps for both existing and proposed conditions are also attached to this report.

<u>Attachments</u>

Attachment A – United States Geological Survey Location Map
Attachment B – Federal Emergency Management Agency Flood Insurance Rate Map

Attachment C – Natural Resources Conservation Service Hydrologic Soil Group Map
Attachment D – Storm Drainage Computations
Attachment E – Water Quality Computations
Attachment F – Hydrologic Analysis – Input Computations
Attachment G – Hydrologic Analysis – Computer Model Results
Attachment H – Watershed Maps

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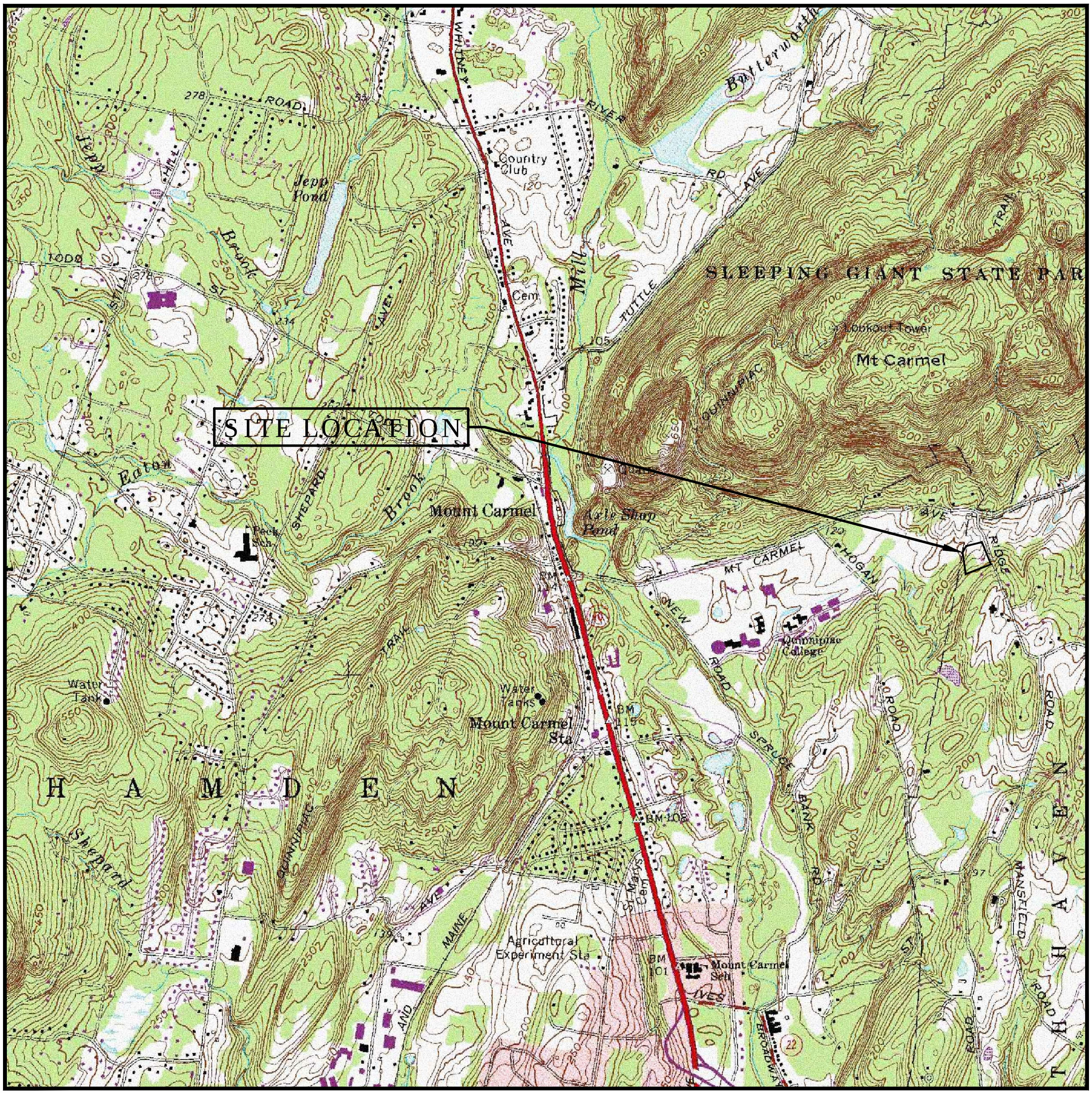
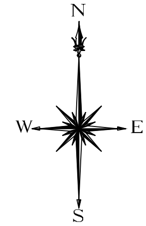
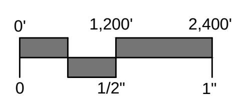
ATTACHMENT A

UNITED STATES GEOLOGICAL SURVEY LOCATION MAP







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USGS QUADRANGLE MAP, QUAD NO. 80

SLATE UPPER SCHOOL

5100 RIDGE ROAD
NORTH HAVEN, CONNECTICUT

PROJECT PHASE:

REV: ---

DATE	OCT. 27, 2020
SCALE	1"=2400'
PROJ. NO.	6156-03

DESIGNED	DRAWN	CHECKED
---	FAB	---

DRAWING NAME:

LOC

Drawing: W:\CADDESIGN\6156-03-DE\CAD\NONPLANSET\USGS QUAD MAP.DWG Layout Tab:LOC

Plotted by: FABIANOB On this date: Sat, 2020 October 24 - 11:03am

ATTACHMENT B

FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD INSURANCE RATE MAP

National Flood Hazard Layer FIRMette

FEMA

72°53'16"W 41°25'31"N

72°52'39"W 41°25'4"N

Feet

0 250 500 1,000 1,500 2,000

1:6,000

Legend

SEE FIS REPORT FOR DETAILED LEGEND AND INDEX MAP FOR FIRM PANEL LAYOUT

SPECIAL FLOOD HAZARD AREAS
- Without Base Flood Elevation (BFE) *Zone A, V, A99*
- With BFE or Depth *Zone AE, AO, AH, VE, AR*
- Regulatory Floodway

OTHER AREAS OF FLOOD HAZARD
- 0.2% Annual Chance Flood Hazard, Areas of 1% annual chance flood with average depth less than one foot or with drainage areas of less than one square mile *Zone X*
- Future Conditions 1% Annual Chance Flood Hazard *Zone X*
- Area with Reduced Flood Risk due to Levee. See Notes. *Zone X*
- Area with Flood Risk due to Levee *Zone D*

OTHER AREAS
- NO SCREEN Area of Minimal Flood Hazard *Zone X*
- Effective LOMRs
- Area of Undetermined Flood Hazard *Zone D*

GENERAL STRUCTURES
- Channel, Culvert, or Storm Sewer
- Levee, Dike, or Floodwall

OTHER FEATURES
- 20.2 / 17.5 Cross Sections with 1% Annual Chance Water Surface Elevation
- Coastal Transect
- 513 Base Flood Elevation Line (BFE)
- Limit of Study
- Jurisdiction Boundary
- Coastal Transect Baseline
- Profile Baseline
- Hydrographic Feature

MAP PANELS
- Digital Data Available
- No Digital Data Available
- Unmapped

N

The pin displayed on the map is an approximate point selected by the user and does not represent an authoritative property location.

This map complies with FEMA's standards for the use of digital flood maps if it is not void as described below. The basemap shown complies with FEMA's basemap accuracy standards

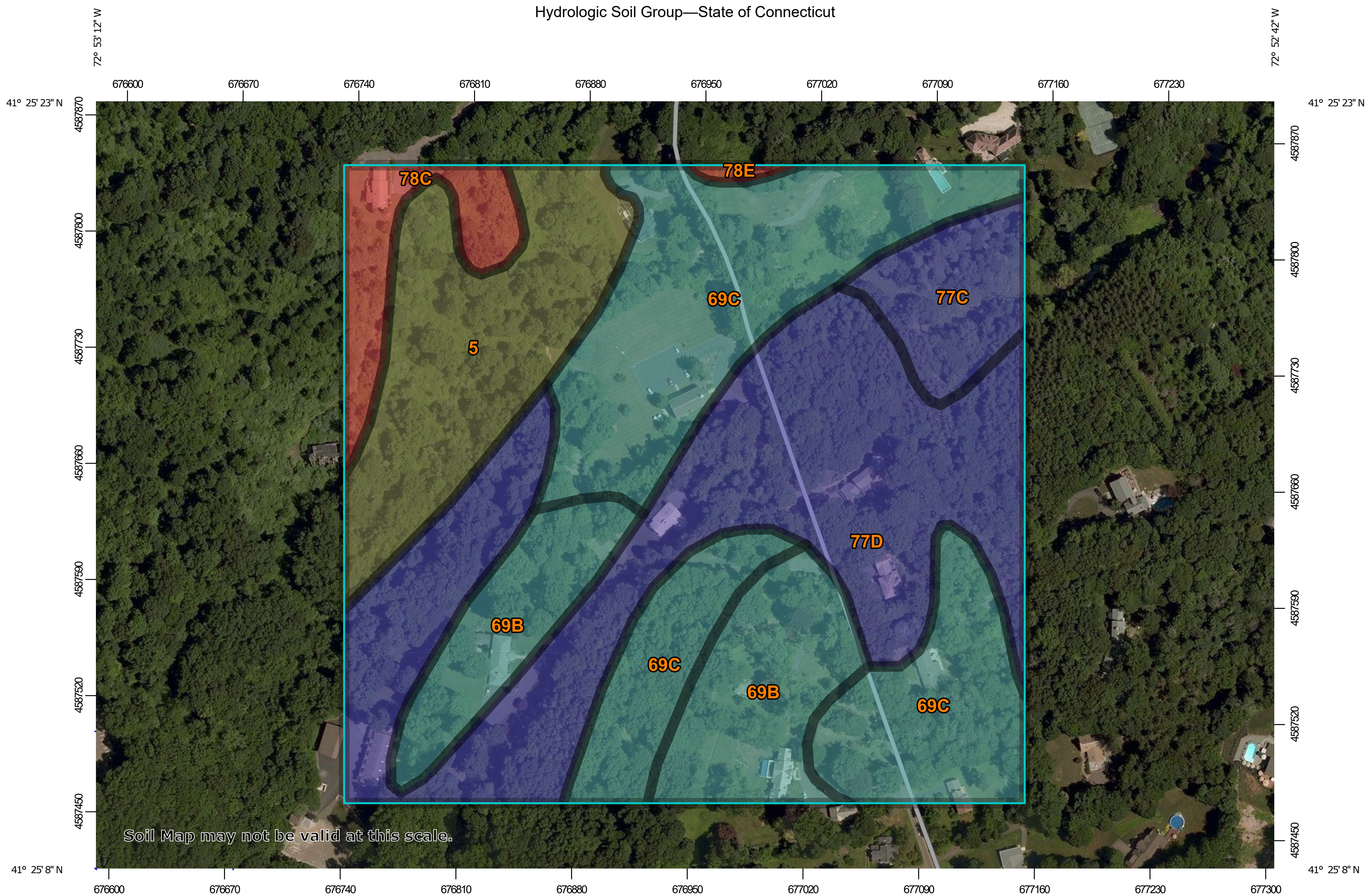
The flood hazard information is derived directly from the authoritative NFHL web services provided by FEMA. This map was exported on 10/9/2020 at 12:33 PM and does not reflect changes or amendments subsequent to this date and time. The NFHL and effective information may change or become superseded by new data over time.

This map image is void if the one or more of the following map elements do not appear: basemap imagery, flood zone labels, legend, scale bar, map creation date, community identifiers, FIRM panel number, and FIRM effective date. Map images for unmapped and unmodernized areas cannot be used for regulatory purposes.

ATTACHMENT C

NATURAL RESOURCES CONSERVATION SERVICE HYDROLOGIC SOIL GROUP MAP

Hydrologic Soil Group—State of Connecticut



Map Scale: 1:3,260 if printed on A landscape (11" x 8.5") sheet.

N

Meters: 0, 45, 90, 180, 270

Feet: 0, 150, 300, 600, 900

Map projection: Web Mercator Corner coordinates: WGS84 Edge tics: UTM Zone 18N WGS84

MAP LEGEND

Area of Interest (AOI)

- Area of Interest (AOI)

Soils

Soil Rating Polygons

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Lines

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Soil Rating Points

- A
- A/D
- B
- B/D
- C
- C/D
- D
- Not rated or not available

Water Features

- Streams and Canals

Transportation

- Rails
- Interstate Highways
- US Routes
- Major Roads
- Local Roads

Background

- Aerial Photography

MAP INFORMATION

The soil surveys that comprise your AOI were mapped at 1:12,000.

Warning: Soil Map may not be valid at this scale.

Enlargement of maps beyond the scale of mapping can cause misunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small areas of contrasting soils that could have been shown at a more detailed scale.

Please rely on the bar scale on each map sheet for map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL:
Coordinate System: Web Mercator (EPSG:3857)

Maps from the Web Soil Survey are based on the Web Mercator projection, which preserves direction and shape but distorts distance and area. A projection that preserves area, such as the Albers equal-area conic projection, should be used if more accurate calculations of distance or area are required.

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: State of Connecticut
Survey Area Data: Version 19, Sep 13, 2019

Soil map units are labeled (as space allows) for map scales 1:50,000 or larger.

Date(s) aerial images were photographed: Jun 27, 2014—Jul 22, 2014

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

Hydrologic Soil Group

Map unit symbol	Map unit name	Rating	Acres in AOI	Percent of AOI
5	Wilbraham silt loam, 0 to 3 percent slopes	C/D	5.5	14.1%
69B	Yalesville fine sandy loam, 3 to 8 percent slopes	C	5.5	13.9%
69C	Yalesville fine sandy loam, 8 to 15 percent slopes	C	12.0	30.5%
77C	Cheshire-Holyoke complex, 3 to 15 percent slopes, very rocky	B	2.0	5.1%
77D	Cheshire-Holyoke complex, 15 to 35 percent slopes, very rocky	B	12.4	31.5%
78C	Holyoke-Rock outcrop complex, 3 to 15 percent slopes	D	1.8	4.6%
78E	Holyoke-Rock outcrop complex, 15 to 45 percent slopes	D	0.1	0.3%
Totals for Area of Interest			**39.3**	**100.0%**

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified

Tie-break Rule: Higher

ATTACHMENT D

STORM DRAINAGE COMPUTATIONS

Rational Method Individual Basin Calculations

Project: Slate Upper School   By: AWG   Date: 10/23/20

Location: 5100 Ridge Road, North Haven, CT   Checked: FAB   Date: 10/25/20

Basin Name	Impervious Area C=0.9 (sf)	Grassed Area C=0.3 (sf)	Wooded Area C=0.2 (sf)	Total Area (sf)	Total Area (ac)	Weighted C	Tc (min)
System 110							
AD 7	2792	4372	0	7164	0.16	0.53	5.0
AD 7A	813	273	0	1085	0.02	0.75	5.0
CB 8	3379	7700	1276	12355	0.28	0.45	5.0
AD 9	402	150	0	552	0.01	0.74	5.0
AD 10	1013	182	0	1195	0.03	0.81	5.0
MH 11	703	0	0	703	0.02	0.90	5.0
AD 12	1053	110	0	1163	0.03	0.84	5.0
AD 13	3854	144	0	3998	0.09	0.88	5.0
AD 14	1024	415	0	1439	0.03	0.73	5.0
MH 15	3283	781	0	4064	0.09	0.78	5.0
AD 17	741	3563	0	4303	0.10	0.40	5.0
CB 18	2362	0	0	2362	0.05	0.90	5.0
CB 19	5132	177	0	5309	0.12	0.88	5.0
CB 20	9988	22426	2342	34756	0.80	0.47	12.5
CB 21	4422	415	0	4837	0.11	0.85	5.0
AD 22	74	292	0	365	0.01	0.42	5.0
AD 23	96	543	0	639	0.01	0.39	5.0
Outlet System 110							
AD 25	0	3564	2198	5763	0.13	0.26	5.0
AD 26	0	1281	1687	2968	0.07	0.24	5.0
AD 27	0	28224	3962	32185	0.74	0.29	10.0
AD 28	0	29919	2783	32702	0.75	0.29	12.5

Time of Concentration (T_c) or Travel Time (T_t) Worksheet

Project:	Slate Upper School	By:	FAB	Date:	10/27/20
Location:	5100 Ridge Road, North Haven, CT	Checked:		Date:	
Circle one:	Present ***Developed***	Watershed:	AD 28		
Circle one:	***T_c*** T_t	Subwatershed:			

Sheet flow (applicable to T_c only)

	Units	Segment ID A-B		Total
1. Surface description (Table 3-1)		WOODS		
2. Manning's roughness coeff. for sheet flow, n (Table 3-1)		0.400		
3. Flow Length, L (< 300ft)	ft.	70.0		
4. Two-year 24-hr rainfall, P_2	in.	3.50		
5. Land slope, s	ft./ft.	0.045		
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}(s^{0.4})}$	hr.	0.186	=	0.186

Shallow concentrated flow (assume hyd. radius = depth of flow)

Segment ID	Units	B-C		C-D		D-E		E-F		Total
7. Surface description		WOODS		BIT		WOODS		GRASS		
8. Manning's roughness coeff., n		0.100		0.015		0.100		0.080		
9. Paved or unpaved		UNPVD		PVD		UNPVD		UNPVD		
10. Depth of flow, d (default values: d=.4 unpaved, d=.2 paved)	ft.	0.40		0.20		0.40		0.40		
11. Flow Length, L	ft.	210.0		101.0		25.0		10.0		
12. Watercourse slope, s	ft./ft.	0.15		0.16		0.16		0.30		
13. Average velocity, $V = \frac{1.49}{n}(d^{2/3})(s^{1/2})$	fps.	3.13		13.59		3.24		5.54		
14. $T_t = \frac{L}{3600 * V}$	hr.	0.019	+	0.002	+	0.002	+	0.001	=	0.023

Channel flow

Segment ID	Units									Total
15. Channel Bottom width, b	ft.									
16. Horizontal side slope component, z (z horiz:1 vert)	ft.									
17. Depth of flow, d	ft.									
18. Cross sectional flow area, A (assume trapazoidal)	$ft.^2$									
19. Wetted perimeter, P_w	ft.									
20. Hydraulic Radius, $R = \frac{A}{P_w}$	ft.									
21. Channel slope, s	ft./ft.									
22. Manning's roughness coeff., n										
23. $V = \frac{1.49}{n}(R^{2/3})(s^{1/2})$	fps.									
24. Flow length, L	ft.									
25. $T_t = \frac{L}{3600 * V}$	hr.							+	=	0.000

26. Watershed or subarea T_c or T_t (add T_t in steps 6, 14 & 25) hr. 0.209

Time of Concentration (T_c) or Travel Time (T_t) Worksheet

Project: Slate Upper School — By: FAB — Date: 10/27/20
Location: 5100 Ridge Road, North Haven, CT — Checked: ______ — Date: ______
Circle one: Present **Developed** — Watershed: CCB 20
Circle one: **T_c** T_t — Subwatershed: ______

Sheet flow (applicable to T_c only)

		Segment ID	A-B		
1.	Surface description (Table 3-1)		WOODS		
2.	Manning's roughness coeff. for sheet flow, n (Table 3-1)		0.400		
3.	Flow Length, L (< 300ft)	ft.	65.0		
4.	Two-year 24-hr rainfall, P_2	in.	3.50		
5.	Land slope, s	ft./ft.	0.045		
6.	$T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}(s^{0.4})}$	hr.	0.175	=	0.175

Shallow concentrated flow (assume hyd. radius = depth of flow)

		Segment ID	B-C	C-D	D-E	E-F	F-G
7.	Surface description		WOODS	BIT	WOODS	GRASS	BIT
8.	Manning's roughness coeff., n		0.100	0.015	0.100	0.080	0.015
9.	Paved or unpaved		UNPVD	PVD	UNPVD	UNPVD	PVD
10.	Depth of flow, d (default values: d=.4 unpaved, d=.2 paved)	ft.	0.40	0.20	0.40	0.40	0.20
11.	Flow Length, L	ft.	270.0	38.0	25.0	40.0	100.0
12.	Watercourse slope, s	ft./ft.	0.17	0.16	0.16	0.20	0.03
13.	Average velocity, $V = \frac{1.49}{n}(d^{2/3})(s^{1/2})$	fps.	3.34	13.59	3.24	4.52	5.88
14.	$T_t = \frac{L}{3600 * V}$	hr.	0.022 +	0.001 +	0.002 +	0.002 +	0.005
						=	0.033

Channel flow

		Segment ID					
15.	Channel Bottom width, b	ft.					
16.	Horizontal side slope component, z (z horiz:1 vert)	ft.					
17.	Depth of flow, d	ft.					
18.	Cross sectional flow area, A (assume trapazoidal)	$ft.^2$					
19.	Wetted perimeter, P_w	ft.					
20.	Hydraulic Radius, $R = \frac{A}{P_w}$	ft.					
21.	Channel slope, s	ft./ft.					
22.	Manning's roughness coeff., n						
23.	$V = \frac{1.49}{n}(R^{2/3})(s^{1/2})$	fps.					
24.	Flow length, L	ft.					
25.	$T_t = \frac{L}{3600 * V}$	hr.				+	= 0.000
26.	Watershed or subarea T_c or T_t (add T_t in steps 6, 14 & 25)	hr.					0.208

NOAA Atlas 14, Volume 10, Version 3
Location name: North Haven, Connecticut, USA*
Latitude: 41.4214°, Longitude: -72.8826°
Elevation: 181.97 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

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NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches/hour)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	**4.09** (3.14-5.15)	**4.96** (3.80-6.24)	**6.37** (4.86-8.05)	**7.54** (5.72-9.59)	**9.14** (6.74-12.2)	**10.3** (7.50-14.2)	**11.6** (8.21-16.6)	**13.1** (8.76-19.1)	**15.2** (9.82-23.1)	**16.9** (10.7-26.3)
10-min	**2.90** (2.23-3.65)	**3.51** (2.69-4.42)	**4.51** (3.44-5.71)	**5.33** (4.06-6.79)	**6.47** (4.78-8.66)	**7.33** (5.31-10.0)	**8.23** (5.81-11.8)	**9.25** (6.20-13.5)	**10.7** (6.95-16.3)	**12.0** (7.57-18.6)
15-min	**2.28** (1.74-2.86)	**2.75** (2.11-3.47)	**3.53** (2.70-4.46)	**4.18** (3.18-5.32)	**5.08** (3.75-6.79)	**5.74** (4.17-7.88)	**6.45** (4.56-9.22)	**7.26** (4.86-10.6)	**8.42** (5.45-12.8)	**9.38** (5.94-14.6)
30-min	**1.58** (1.21-1.99)	**1.91** (1.46-2.40)	**2.44** (1.87-3.09)	**2.89** (2.20-3.68)	**3.50** (2.59-4.69)	**3.96** (2.87-5.43)	**4.45** (3.15-6.36)	**5.01** (3.35-7.33)	**5.81** (3.76-8.85)	**6.48** (4.10-10.1)
60-min	**1.01** (0.775-1.27)	**1.22** (0.935-1.54)	**1.56** (1.19-1.97)	**1.85** (1.40-2.35)	**2.24** (1.65-2.99)	**2.53** (1.83-3.47)	**2.84** (2.01-4.06)	**3.19** (2.14-4.67)	**3.71** (2.40-5.64)	**4.13** (2.62-6.44)
2-hr	**0.666** (0.514-0.832)	**0.795** (0.614-0.994)	**1.01** (0.774-1.26)	**1.18** (0.905-1.50)	**1.42** (1.06-1.89)	**1.61** (1.17-2.19)	**1.80** (1.28-2.55)	**2.02** (1.36-2.94)	**2.34** (1.52-3.54)	**2.61** (1.66-4.04)
3-hr	**0.515** (0.400-0.641)	**0.614** (0.476-0.765)	**0.776** (0.599-0.970)	**0.910** (0.699-1.15)	**1.10** (0.817-1.45)	**1.23** (0.902-1.67)	**1.38** (0.984-1.95)	**1.55** (1.05-2.25)	**1.80** (1.17-2.71)	**2.01** (1.27-3.09)
6-hr	**0.328** (0.257-0.406)	**0.392** (0.306-0.486)	**0.497** (0.387-0.618)	**0.584** (0.452-0.730)	**0.704** (0.528-0.926)	**0.793** (0.584-1.07)	**0.888** (0.638-1.25)	**1.00** (0.677-1.44)	**1.17** (0.761-1.75)	**1.31** (0.832-2.00)
12-hr	**0.202** (0.158-0.247)	**0.243** (0.191-0.299)	**0.312** (0.244-0.385)	**0.369** (0.287-0.458)	**0.447** (0.338-0.585)	**0.505** (0.374-0.678)	**0.567** (0.410-0.797)	**0.642** (0.436-0.918)	**0.754** (0.494-1.12)	**0.850** (0.544-1.29)
24-hr	**0.119** (0.094-0.145)	**0.146** (0.115-0.178)	**0.190** (0.150-0.233)	**0.227** (0.178-0.279)	**0.277** (0.211-0.361)	**0.314** (0.235-0.421)	**0.355** (0.259-0.498)	**0.405** (0.276-0.575)	**0.482** (0.316-0.712)	**0.549** (0.352-0.829)
2-day	**0.067** (0.053-0.081)	**0.083** (0.066-0.101)	**0.111** (0.088-0.135)	**0.133** (0.105-0.163)	**0.164** (0.126-0.213)	**0.187** (0.141-0.250)	**0.212** (0.156-0.298)	**0.244** (0.167-0.345)	**0.295** (0.194-0.434)	**0.340** (0.219-0.510)
3-day	**0.048** (0.039-0.058)	**0.061** (0.048-0.073)	**0.081** (0.064-0.098)	**0.097** (0.077-0.118)	**0.120** (0.092-0.155)	**0.136** (0.103-0.182)	**0.155** (0.115-0.217)	**0.179** (0.123-0.252)	**0.217** (0.143-0.318)	**0.251** (0.162-0.375)
4-day	**0.039** (0.031-0.047)	**0.049** (0.039-0.059)	**0.065** (0.052-0.078)	**0.078** (0.062-0.095)	**0.096** (0.074-0.124)	**0.109** (0.083-0.145)	**0.124** (0.092-0.173)	**0.143** (0.098-0.201)	**0.173** (0.114-0.253)	**0.200** (0.129-0.298)
7-day	**0.027** (0.021-0.032)	**0.033** (0.026-0.039)	**0.043** (0.034-0.051)	**0.051** (0.041-0.062)	**0.063** (0.049-0.080)	**0.071** (0.054-0.094)	**0.080** (0.060-0.111)	**0.092** (0.063-0.128)	**0.111** (0.073-0.160)	**0.127** (0.082-0.188)
10-day	**0.022** (0.017-0.026)	**0.026** (0.021-0.031)	**0.034** (0.027-0.040)	**0.040** (0.032-0.048)	**0.048** (0.037-0.061)	**0.054** (0.041-0.071)	**0.061** (0.045-0.084)	**0.070** (0.048-0.097)	**0.083** (0.055-0.119)	**0.094** (0.061-0.139)
20-day	**0.015** (0.013-0.018)	**0.018** (0.015-0.021)	**0.022** (0.018-0.026)	**0.025** (0.020-0.030)	**0.030** (0.023-0.037)	**0.033** (0.025-0.043)	**0.037** (0.027-0.049)	**0.041** (0.028-0.056)	**0.047** (0.031-0.067)	**0.052** (0.034-0.076)
30-day	**0.013** (0.011-0.015)	**0.015** (0.012-0.017)	**0.017** (0.014-0.020)	**0.020** (0.016-0.023)	**0.023** (0.018-0.028)	**0.025** (0.019-0.032)	**0.028** (0.020-0.036)	**0.030** (0.021-0.041)	**0.034** (0.023-0.048)	**0.037** (0.024-0.054)
45-day	**0.011** (0.009-0.013)	**0.012** (0.010-0.014)	**0.014** (0.011-0.016)	**0.015** (0.012-0.018)	**0.017** (0.014-0.022)	**0.019** (0.015-0.024)	**0.021** (0.015-0.027)	**0.022** (0.016-0.031)	**0.025** (0.017-0.035)	**0.026** (0.017-0.038)
60-day	**0.009** (0.008-0.011)	**0.010** (0.008-0.012)	**0.012** (0.010-0.014)	**0.013** (0.011-0.015)	**0.015** (0.011-0.018)	**0.016** (0.012-0.020)	**0.017** (0.013-0.022)	**0.018** (0.013-0.025)	**0.020** (0.013-0.028)	**0.021** (0.014-0.030)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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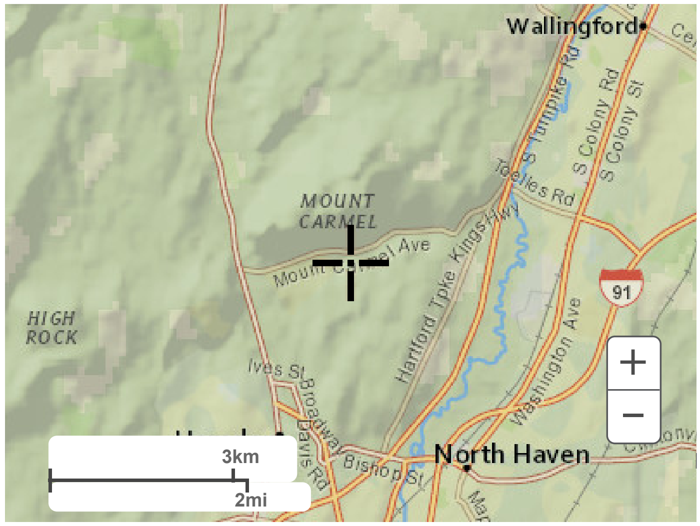
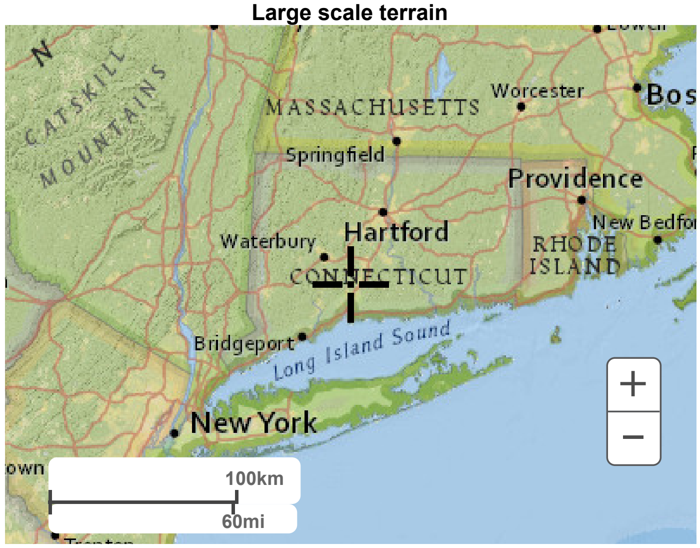
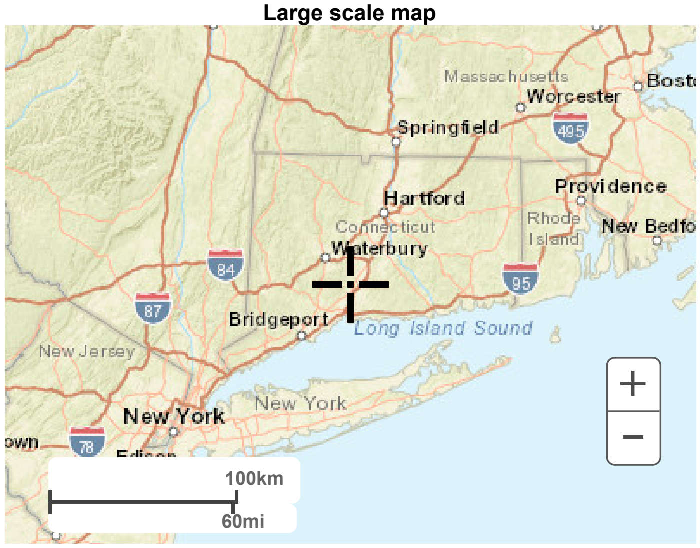
PF graphical



Large scale terrain

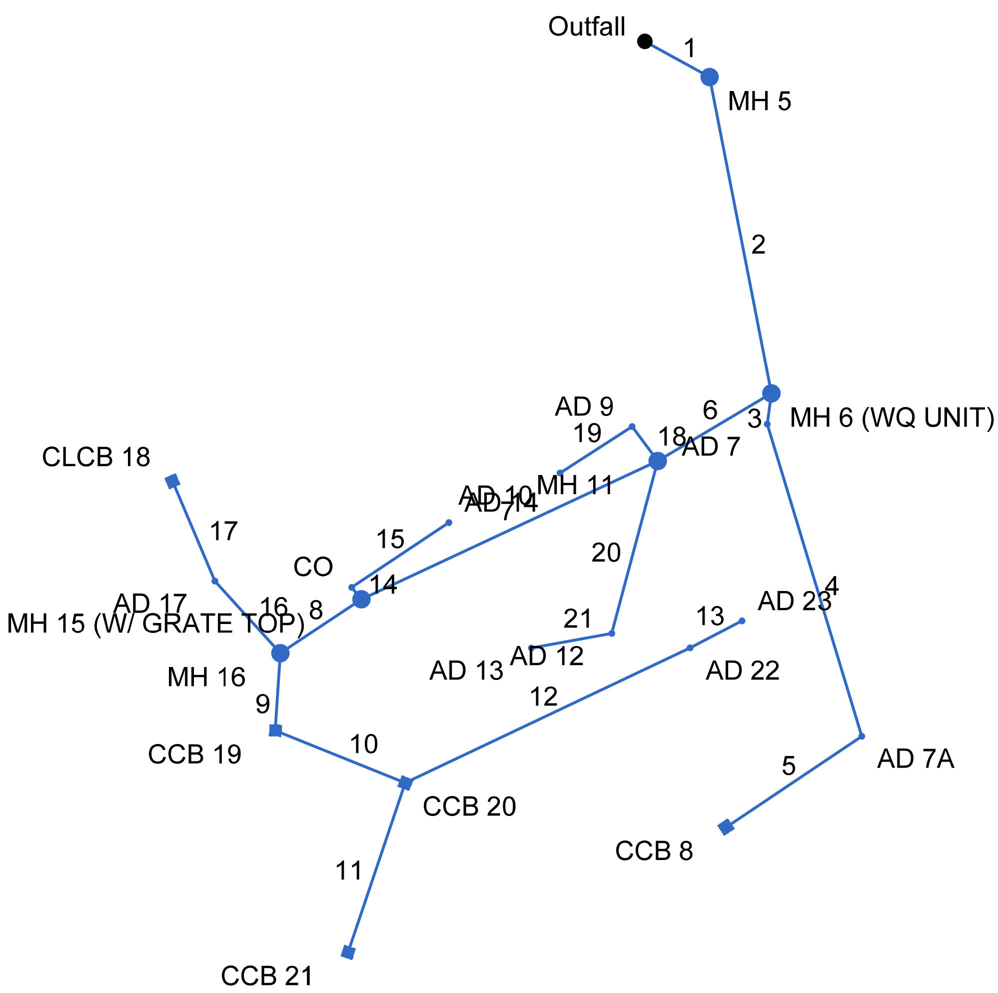


Large scale map



Large scale aerial

Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Project File: System 110.stm	Number of lines: 21	Date: 10/25/2020

Storm Sewers v2018.30

Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data								Line ID
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
1	End	19.000	28.942	MH	0.00	0.00	0.00	5.0	156.00	1.58	156.30	18	Cir	0.012	0.80	164.00	MH 5-FES 4
2	1	83.000	50.011	MH	0.00	0.00	0.00	5.0	160.00	7.71	166.40	18	Cir	0.012	0.95	172.80	MH 6-MH 5
3	2	8.000	18.565	Grate	0.00	0.16	0.53	5.0	167.30	5.00	167.70	12	Cir	0.012	0.70	172.60	AD 7-MH 6
4	3	84.000	-24.256	Grate	0.00	0.02	0.75	5.0	168.50	9.52	176.50	12	Cir	0.012	1.44	183.60	AD 7A-AD 7
5	4	42.000	72.733	Comb	0.00	0.28	0.45	5.0	179.00	2.38	180.00	12	Cir	0.012	1.00	183.00	CB 8-AD 7A
6	2	34.000	70.230	Grate	0.00	0.02	0.90	5.0	166.40	0.88	166.70	18	Cir	0.012	1.67	173.40	MH 11-MH 6
7	6	84.000	5.683	Grate	0.00	0.09	0.78	5.0	167.00	1.79	168.50	15	Cir	0.012	1.46	173.60	MH 15-MH 11
8	7	25.000	-8.463	MH	0.00	0.00	0.00	0.0	168.50	2.80	169.20	15	Cir	0.012	0.99	175.00	MH 16-MH 15
9	8	20.000	-52.585	Comb	0.00	0.12	0.88	5.0	170.50	2.50	171.00	15	Cir	0.012	1.44	177.80	CB 19-MH 16
10	9	36.000	-71.754	Comb	0.00	0.80	0.47	12.5	171.00	1.94	171.70	12	Cir	0.012	1.75	177.80	CB 20-CB 19
11	10	46.000	86.536	Comb	0.00	0.11	0.85	5.0	174.80	3.91	176.60	12	Cir	0.012	1.00	179.60	CB 21-CB 20
12	10	81.000	-47.513	Grate	0.00	0.01	0.42	5.0	172.20	0.62	172.70	6	Cir	0.012	0.50	175.80	AD 22-CB 20
13	12	15.000	-2.097	Grate	0.00	0.01	0.39	5.0	172.70	0.67	172.80	6	Cir	0.012	1.00	175.80	AD 23-AD 22
14	7	4.000	75.852	MH	0.00	0.00	0.00	5.0	170.40	2.50	170.50	8	Cir	0.012	1.00	171.50	CO-MH 15
15	14	30.000	95.545	Grate	0.00	0.03	0.73	5.0	170.50	1.00	170.80	6	Cir	0.012	1.00	173.30	AD 14-CO
16	8	25.000	81.383	Grate	0.00	0.10	0.40	5.0	169.50	1.20	169.80	12	Cir	0.012	0.57	172.30	AD 17-MH 16
17	16	28.000	19.322	Grate	0.00	0.05	0.90	5.0	169.80	0.71	170.00	12	Cir	0.012	1.00	173.30	CB 18-AD 17
18	6	11.000	84.267	Grate	0.00	0.01	0.74	5.0	169.00	2.73	169.30	8	Cir	0.012	1.50	173.20	AD 9-MH 11
19	18	22.000	-86.223	Grate	0.00	0.03	0.81	5.0	169.90	1.36	170.20	6	Cir	0.012	1.00	173.18	AD 10-AD 9
20	6	46.000	-44.320	Grate	0.00	0.03	0.84	5.0	169.00	1.52	169.70	8	Cir	0.012	1.38	173.40	AD 12-MH 11
21	20	21.000	64.908	Grate	0.00	0.09	0.88	5.0	169.70	2.38	170.20	8	Cir	0.012	1.00	173.20	AD 13-AD 12

Project File: System 110.stm	Number of lines: 21	Date: 10/25/2020

Storm Sewer Tabulation

Station		Len	Drng Area		Rnoff coeff	Area x C		Tc		Rain (I)	Total flow	Cap full	Vel	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr	Total		Incr	Total	Inlet	Syst					Size	Slope	Dn	Up	Dn	Up	Dn	Up	
		(ft)	(ac)	(ac)	(C)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
1	End	19.000	0.00	1.96	0.00	0.00	1.14	5.0	13.3	5.7	6.47	14.30	4.00	18	1.58	156.00	156.30	157.50	157.48	156.63	164.00	MH 5-FES 4
2	1	83.000	0.00	1.96	0.00	0.00	1.14	5.0	13.1	5.7	6.51	31.59	9.67	18	7.71	160.00	166.40	160.46	167.39	164.00	172.80	MH 6-MH 5
3	2	8.000	0.16	0.46	0.53	0.08	0.23	5.0	5.4	8.9	2.00	8.63	6.49	12	5.00	167.30	167.70	167.63	168.30	172.80	172.60	AD 7-MH 6
4	3	84.000	0.02	0.30	0.75	0.02	0.14	5.0	5.2	9.0	1.27	11.91	6.66	12	9.52	168.50	176.50	168.72	176.98	172.60	183.60	AD 7A-AD 7
5	4	42.000	0.28	0.28	0.45	0.13	0.13	5.0	5.0	9.1	1.15	5.95	4.60	12	2.38	179.00	180.00	179.30	180.45	183.60	183.00	CB 8-AD 7A
6	2	34.000	0.02	1.50	0.90	0.02	0.91	5.0	13.0	5.7	5.25	10.69	4.56	18	0.88	166.40	166.70	167.39	167.58	172.80	173.40	MH 11-MH 6
7	6	84.000	0.09	1.32	0.78	0.07	0.76	5.0	12.8	5.8	4.40	9.35	6.23	15	1.79	167.00	168.50	167.60	169.35	173.40	173.60	MH 15-MH 11
8	7	25.000	0.00	1.20	0.00	0.00	0.67	0.0	12.7	5.8	3.89	11.71	4.54	15	2.80	168.50	169.20	169.35	170.00	173.60	175.00	MH 16-MH 15
9	8	20.000	0.12	1.05	0.88	0.11	0.58	5.0	12.6	5.8	3.40	11.06	6.20	15	2.50	170.50	171.00	170.98	171.74	175.00	177.80	CB 19-MH 16
10	9	36.000	0.80	0.93	0.47	0.38	0.48	12.5	12.5	5.9	2.80	5.38	4.56	12	1.94	171.00	171.70	171.74	172.42	177.80	177.80	CB 20-CB 19
11	10	46.000	0.11	0.11	0.85	0.09	0.09	5.0	5.0	9.1	0.85	7.63	4.73	12	3.91	174.80	176.60	175.03	176.99	177.80	179.60	CB 21-CB 20
12	10	81.000	0.01	0.02	0.42	0.00	0.01	5.0	5.2	9.0	0.07	0.48	1.32	6	0.62	172.20	172.70	172.42	172.83	177.80	175.80	AD 22-CB 20
13	12	15.000	0.01	0.01	0.39	0.00	0.00	5.0	5.0	9.1	0.04	0.50	1.15	6	0.67	172.70	172.80	172.83	172.89	175.80	175.80	AD 23-AD 22
14	7	4.000	0.00	0.03	0.00	0.00	0.02	5.0	5.2	9.0	0.20	2.07	2.95	8	2.50	170.40	170.50	170.54	170.70	173.60	171.50	CO-MH 15
15	14	30.000	0.03	0.03	0.73	0.02	0.02	5.0	5.0	9.1	0.20	0.61	2.50	6	1.00	170.50	170.80	170.70	171.02	171.50	173.30	AD 14-CO
16	8	25.000	0.10	0.15	0.40	0.04	0.09	5.0	5.2	8.9	0.76	4.23	2.45	12	1.20	169.50	169.80	170.00	170.16	175.00	172.30	AD 17-MH 16
17	16	28.000	0.05	0.05	0.90	0.05	0.05	5.0	5.0	9.1	0.41	3.26	2.03	12	0.71	169.80	170.00	170.16	170.26	172.30	173.30	CB 18-AD 17
18	6	11.000	0.01	0.04	0.74	0.01	0.03	5.0	5.1	9.0	0.29	2.16	3.36	8	2.73	169.00	169.30	169.16	169.55	173.40	173.20	AD 9-MH 11
19	18	22.000	0.03	0.03	0.81	0.02	0.02	5.0	5.0	9.1	0.22	0.71	2.81	6	1.36	169.90	170.20	170.09	170.44	173.20	173.18	AD 10-AD 9
20	6	46.000	0.03	0.12	0.84	0.03	0.10	5.0	5.1	9.0	0.94	1.61	4.23	8	1.52	169.00	169.70	169.37	170.16	173.40	173.40	AD 12-MH 11
21	20	21.000	0.09	0.09	0.88	0.08	0.08	5.0	5.0	9.1	0.72	2.02	3.05	8	2.38	169.70	170.20	170.16	170.60	173.40	173.20	AD 13-AD 12

Project File: System 110.stm

Number of lines: 21

Run Date: 10/25/2020

NOTES:Intensity = 40.58 / (Inlet time + 3.50) ^ 0.70; Return period =Yrs. 25 ; c = cir e = ellip b = box

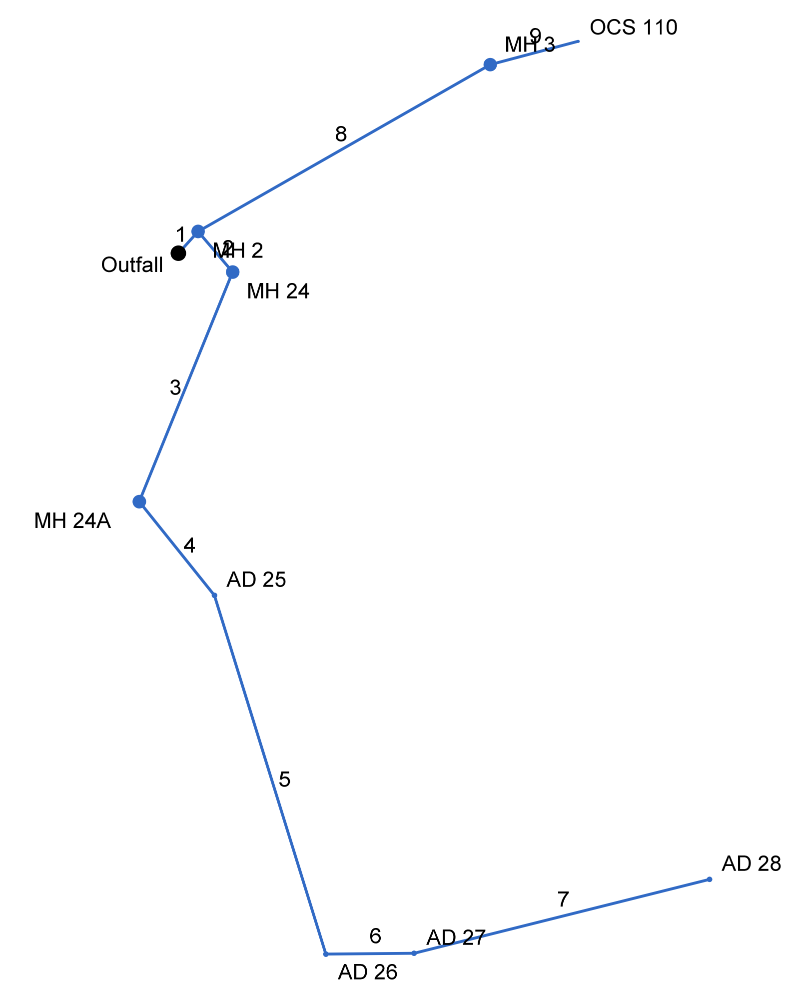
Hydraulic Grade Line Computations

Line	Size	Q	Downstream								Len	Upstream								Check		JL coeff	Minor loss
	(in)	(cfs)	Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	(ft)	Invert elev (ft)	HGL elev (ft)	Depth (ft)	Area (sqft)	Vel (ft/s)	Vel head (ft)	EGL elev (ft)	Sf (%)	Ave Sf (%)	Enrgy loss (ft)	(K)	(ft)
1	18	6.47	156.00	157.50	1.50	1.77	3.66	0.21	157.71	0.323	19.000	156.30	157.48	1.18	1.49	4.34	0.29	157.77	0.350	0.337	0.064	0.80	0.23
2	18	6.51	160.00	160.46	0.46*	0.46	14.06	0.43	160.90	0.000	83.000	166.40	167.39	0.99**	1.23	5.29	0.43	167.82	0.000	0.000	n/a	0.95	0.41
3	12	2.00	167.30	167.63	0.33*	0.22	8.94	0.25	167.88	0.000	8.000	167.70	168.30	0.60**	0.49	4.04	0.25	168.56	0.000	0.000	n/a	0.70	0.18
4	12	1.27	168.50	168.72	0.22*	0.13	9.86	0.18	168.91	0.000	84.000	176.50	176.98	0.48**	0.37	3.45	0.18	177.16	0.000	0.000	n/a	1.44	0.27
5	12	1.15	179.00	179.30	0.30*	0.20	5.85	0.17	179.47	0.000	42.000	180.00	180.45	0.45**	0.34	3.34	0.17	180.62	0.000	0.000	n/a	1.00	n/a
6	18	5.25	166.40	167.39	0.99	1.08	4.26	0.37	167.75	0.000	34.000	166.70	167.58 j	0.88**	1.08	4.86	0.37	167.95	0.000	0.000	n/a	1.67	n/a
7	15	4.40	167.00	167.60	0.60*	0.59	7.50	0.38	167.99	0.000	84.000	168.50	169.35	0.85**	0.89	4.96	0.38	169.73	0.000	0.000	n/a	1.46	n/a
8	15	3.89	168.50	169.35	0.85	0.83	4.37	0.34	169.69	0.000	25.000	169.20	170.00 j	0.80**	0.83	4.71	0.34	170.34	0.000	0.000	n/a	0.99	n/a
9	15	3.40	170.50	170.98	0.48*	0.43	7.93	0.31	171.29	0.000	20.000	171.00	171.74	0.74**	0.76	4.47	0.31	172.05	0.000	0.000	n/a	1.44	n/a
10	12	2.80	171.00	171.74	0.74	0.60	4.47	0.34	172.08	0.000	36.000	171.70	172.42 j	0.72**	0.60	4.65	0.34	172.75	0.000	0.000	n/a	1.75	n/a
11	12	0.85	174.80	175.03	0.23*	0.13	6.41	0.14	175.17	0.000	46.000	176.60	176.99	0.39**	0.28	3.05	0.14	177.13	0.000	0.000	n/a	1.00	0.14
12	6	0.07	172.20	172.42	0.22	0.04	0.89	0.05	172.46	0.000	81.000	172.70	172.83	0.13**	0.04	1.74	0.05	172.88	0.000	0.000	n/a	0.50	n/a
13	6	0.04	172.70	172.83	0.13	0.02	0.86	0.03	172.86	0.000	15.000	172.80	172.89	0.09**	0.02	1.44	0.03	172.92	0.000	0.000	n/a	1.00	n/a
14	8	0.20	170.40	170.54	0.14*	0.05	3.73	0.07	170.61	0.000	4.000	170.50	170.70	0.20**	0.09	2.18	0.07	170.78	0.000	0.000	n/a	1.00	0.07
15	6	0.20	170.50	170.70	0.20	0.08	2.66	0.09	170.79	0.000	30.000	170.80	171.02	0.22**	0.09	2.35	0.09	171.11	0.000	0.000	n/a	1.00	0.09
16	12	0.76	169.50	170.00	0.50	0.26	1.95	0.13	170.13	0.000	25.000	169.80	170.16 j	0.36**	0.26	2.94	0.13	170.30	0.000	0.000	n/a	0.57	0.08
17	12	0.41	169.80	170.16	0.36	0.17	1.59	0.09	170.26	0.000	28.000	170.00	170.26 j	0.26**	0.17	2.47	0.09	170.36	0.000	0.000	n/a	1.00	n/a
18	8	0.29	169.00	169.16	0.16*	0.07	4.29	0.09	169.26	0.000	11.000	169.30	169.55	0.25**	0.12	2.43	0.09	169.64	0.000	0.000	n/a	1.50	n/a
19	6	0.22	169.90	170.09	0.19*	0.07	3.19	0.09	170.18	0.000	22.000	170.20	170.44	0.24**	0.09	2.43	0.09	170.53	0.000	0.000	n/a	1.00	0.09
20	8	0.94	169.00	169.37	0.37*	0.20	4.80	0.21	169.58	0.000	46.000	169.70	170.16	0.46**	0.26	3.67	0.21	170.37	0.000	0.000	n/a	1.38	0.29
21	8	0.72	169.70	170.16	0.46	0.22	2.81	0.17	170.33	0.000	21.000	170.20	170.60 j	0.40**	0.22	3.29	0.17	170.77	0.000	0.000	n/a	1.00	0.17

Project File: System 110.stm	Number of lines: 21	Run Date: 10/25/2020

Notes: * depth assumed; ** Critical depth.; j-Line contains hyd. jump ; c = cir e = ellip b = box

Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Project File: Outlet 110 - 25 year.stm	Number of lines: 9	Date: 10/25/2020

Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data								Line ID
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
1	End	11.000	-48.004	MH	0.00	0.00	0.00	5.0	150.00	2.73	150.30	18	Cir	0.012	1.00	154.00	MH 2-FES 1
2	1	20.000	97.696	MH	0.00	0.00	0.00	5.0	150.30	1.50	150.60	12	Cir	0.012	0.91	154.00	MH 24-MH 2
3	2	93.000	62.369	MH	0.00	0.00	0.00	5.0	150.60	1.51	152.00	12	Cir	0.012	0.89	155.00	MH 24A-MH 24
4	3	45.000	-60.717	Grate	0.00	0.13	0.26	5.0	152.00	10.00	156.50	12	Cir	0.012	0.63	165.00	AD 25-MH 24A
5	4	141.000	21.450	Grate	0.00	0.07	0.24	5.0	162.00	9.93	176.00	12	Cir	0.012	1.45	184.30	AD 26-AD 27
6	5	33.000	-73.271	Grate	0.00	0.74	0.29	10.0	180.50	2.12	181.20	12	Cir	0.012	0.50	185.60	AD 27-AD 26
7	6	114.000	-13.635	Grate	0.00	0.75	0.29	12.5	181.20	1.14	182.50	12	Cir	0.012	1.00	186.20	AD 28-AD 27
8	1	126.000	18.192	MH	0.00	0.00	0.00	5.0	150.50	1.59	152.50	18	Cir	0.012	0.30	159.00	MH 3-MH 2
9	8	34.000	14.919	None	8.80	0.00	0.00	5.0	152.50	1.47	153.00	18	Cir	0.012	1.00	158.42	OCS 110-MH 3

Project File: Outlet 110 - 25 year.stm	Number of lines: 9	Date: 10/25/2020

Storm Sewer Tabulation

Station		Len	Drng Area		Rnoff coeff	Area x C		Tc		Rain (I)	Total flow	Cap full	Vel	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line		Incr	Total		Incr	Total	Inlet	Syst					Size	Slope	Dn	Up	Dn	Up	Dn	Up	
		(ft)	(ac)	(ac)	(C)			(min)	(min)	(in/hr)	(cfs)	(cfs)	(ft/s)	(in)	(%)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	
1	End	11.000	0.00	1.69	0.00	0.00	0.48	5.0	14.2	5.5	11.44	18.79	6.78	18	2.73	150.00	150.30	151.50	151.59	151.50	154.00	MH 2-FES 1
2	1	20.000	0.00	1.69	0.00	0.00	0.48	5.0	14.1	5.5	2.65	4.73	3.37	12	1.50	150.30	150.60	151.59	151.68	154.00	154.00	MH 24-MH 2
3	2	93.000	0.00	1.69	0.00	0.00	0.48	5.0	13.7	5.6	2.69	4.73	3.99	12	1.51	150.60	152.00	151.84	152.70	154.00	155.00	MH 24A-MH 24
4	3	45.000	0.13	1.69	0.26	0.03	0.48	5.0	13.5	5.6	2.71	12.20	4.59	12	10.00	152.00	156.50	152.70	157.20	155.00	165.00	AD 25-MH 24A
5	4	141.000	0.07	1.56	0.24	0.02	0.45	5.0	13.3	5.7	2.55	12.16	8.34	12	9.93	162.00	176.00	162.31	176.68	165.00	184.30	AD 26-AD 27
6	5	33.000	0.74	1.49	0.29	0.21	0.43	10.0	13.2	5.7	2.46	5.62	5.65	12	2.12	180.50	181.20	180.96	181.87	184.30	185.60	AD 27-AD 26
7	6	114.000	0.75	0.75	0.29	0.22	0.22	12.5	12.5	5.9	1.27	4.12	2.86	12	1.14	181.20	182.50	181.87	182.98	185.60	186.20	AD 28-AD 27
8	1	126.000	0.00	0.00	0.00	0.00	0.00	5.0	5.1	0.0	8.80	14.33	6.23	18	1.59	150.50	152.50	151.59	153.65	154.00	159.00	MH 3-MH 2
9	8	34.000	0.00	0.00	0.00	0.00	0.00	5.0	5.0	0.0	8.80	13.80	6.07	18	1.47	152.50	153.00	153.65	154.15	159.00	158.42	OCS 110-MH 3

Project File: Outlet 110 - 25 year.stm | Number of lines: 9 | Run Date: 10/25/2020

NOTES:Intensity = 40.58 / (Inlet time + 3.50) ^ 0.70; Return period =Yrs. 25 ; c = cir e = ellip b = box

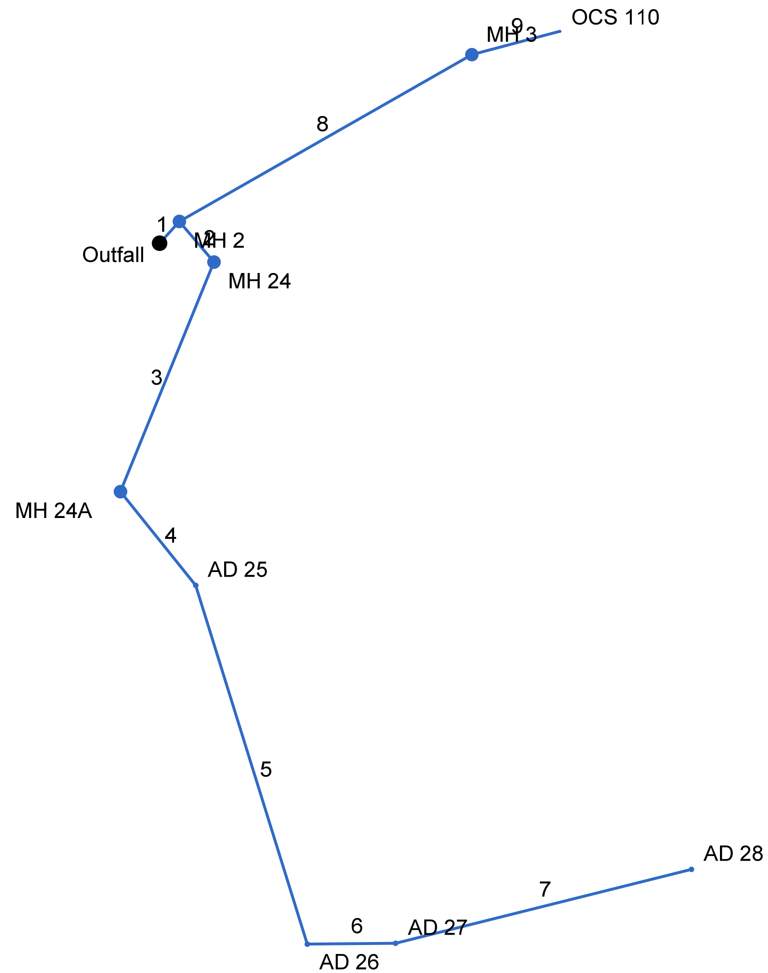
Hydraulic Grade Line Computations

Line	Size	Q	Downstream								Len	Upstream								Check		JL coeff	Minor loss
			Invert elev	HGL elev	Depth	Area	Vel	Vel head	EGL elev	Sf		Invert elev	HGL elev	Depth	Area	Vel	Vel head	EGL elev	Sf	Ave Sf	Enrgy loss		
	(in)	(cfs)	(ft)	(ft)	(ft)	(sqft)	(ft/s)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)	(sqft)	(ft/s)	(ft)	(ft)	(%)	(%)	(ft)	(K)	(ft)
1	18	11.44	150.00	151.50	1.50	1.62	6.47	0.65	152.15	1.011	11.000	150.30	151.59 j	1.29**	1.62	7.08	0.78	152.37	0.937	0.974	n/a	1.00	n/a
2	12	2.65	150.30	151.59	1.00	0.79	3.37	0.18	151.77	0.471	20.000	150.60	151.68	1.00	0.79	3.37	0.18	151.86	0.471	0.471	0.094	0.91	0.16
3	12	2.69	150.60	151.84	1.00	0.59	3.43	0.18	152.03	0.486	93.000	152.00	152.70 j	0.70**	0.59	4.56	0.32	153.03	0.687	0.587	n/a	0.89	n/a
4	12	2.71	152.00	152.70	0.70	0.59	4.59	0.33	153.03	0.000	45.000	156.50	157.20	0.70**	0.59	4.58	0.33	157.53	0.000	0.000	n/a	0.63	0.21
5	12	2.55	162.00	162.31	0.31*	0.21	12.23	0.31	162.62	0.000	141.000	176.00	176.68	0.68**	0.57	4.46	0.31	176.99	0.000	0.000	n/a	1.45	0.45
6	12	2.46	180.50	180.96	0.46*	0.36	6.91	0.30	181.26	0.000	33.000	181.20	181.87	0.67**	0.56	4.39	0.30	182.17	0.000	0.000	n/a	0.50	0.15
7	12	1.27	181.20	181.87	0.67	0.37	2.27	0.19	182.06	0.000	114.000	182.50	182.98 j	0.48**	0.37	3.45	0.19	183.16	0.000	0.000	n/a	1.00	n/a
8	18	8.80	150.50	151.59	1.09	1.37	6.40	0.57	152.16	0.000	126.000	152.50	153.65	1.15**	1.45	6.07	0.57	154.22	0.000	0.000	n/a	0.30	n/a
9	18	8.80	152.50	153.65	1.15*	1.45	6.07	0.57	154.22	0.000	34.000	153.00	154.15	1.15**	1.45	6.07	0.57	154.72	0.000	0.000	n/a	1.00	n/a

Project File: Outlet 110 - 25 year.stm	Number of lines: 9	Run Date: 10/25/2020

Notes: * depth assumed; ** Critical depth.; j-Line contains hyd. jump ; c = cir e = ellip b = box

Hydraflow Storm Sewers Extension for Autodesk® AutoCAD® Civil 3D® Plan



Project File: Outlet 110 - 100 year.stm	Number of lines: 9	Date: 10/25/2020

Storm Sewer Inventory Report

Line No.	Alignment				Flow Data				Physical Data								Line ID
	Dnstr Line No.	Line Length (ft)	Defl angle (deg)	Junc Type	Known Q (cfs)	Drng Area (ac)	Runoff Coeff (C)	Inlet Time (min)	Invert El Dn (ft)	Line Slope (%)	Invert El Up (ft)	Line Size (in)	Line Shape	N Value (n)	J-Loss Coeff (K)	Inlet/ Rim El (ft)	
1	End	11.000	-48.004	MH	0.00	0.00	0.00	5.0	150.00	2.73	150.30	18	Cir	0.012	1.00	154.00	MH 2-FES 1
2	1	20.000	97.696	MH	0.00	0.00	0.00	5.0	150.30	1.50	150.60	12	Cir	0.012	0.91	154.00	MH 24-MH 2
3	2	93.000	62.369	MH	0.00	0.00	0.00	5.0	150.60	1.51	152.00	12	Cir	0.012	0.89	155.00	MH 24A-MH 24
4	3	45.000	-60.717	Grate	0.00	0.13	0.26	5.0	152.00	10.00	156.50	12	Cir	0.012	0.63	165.00	AD 25-MH 24A
5	4	141.000	21.450	Grate	0.00	0.07	0.24	5.0	162.00	9.93	176.00	12	Cir	0.012	1.45	184.30	AD 26-AD 27
6	5	33.000	-73.271	Grate	0.00	0.74	0.29	10.0	180.50	2.12	181.20	12	Cir	0.012	0.50	185.60	AD 27-AD 26
7	6	114.000	-13.635	Grate	0.00	0.75	0.29	12.5	181.20	1.14	182.50	12	Cir	0.012	1.00	186.20	AD 28-AD 27
8	1	126.000	18.192	MH	0.00	0.00	0.00	5.0	150.50	1.59	152.50	18	Cir	0.012	0.30	159.00	MH 3-MH 2
9	8	34.000	14.919	None	13.50	0.00	0.00	5.0	152.50	1.47	153.00	18	Cir	0.012	1.00	158.42	OCS 110-MH 3

Project File: Outlet 110 - 100 year.stm	Number of lines: 9	Date: 10/25/2020

Storm Sewer Tabulation

Station		Len	Drng Area		Rnoff coeff	Area x C		Tc		Rain (I)	Total flow	Cap full	Vel	Pipe		Invert Elev		HGL Elev		Grnd / Rim Elev		Line ID
Line	To Line	(ft)	Incr (ac)	Total (ac)	(C)	Incr	Total	Inlet (min)	Syst (min)	(in/hr)	(cfs)	(cfs)	(ft/s)	Size (in)	Slope (%)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	Dn (ft)	Up (ft)	
1	End	11.000	0.00	1.69	0.00	0.00	0.48	5.0	14.0	7.0	16.88	18.79	9.62	18	2.73	150.00	150.30	151.50	151.74	151.50	154.00	MH 2-FES 1
2	1	20.000	0.00	1.69	0.00	0.00	0.48	5.0	13.9	7.0	3.39	4.73	4.32	12	1.50	150.30	150.60	151.74	151.89	154.00	154.00	MH 24-MH 2
3	2	93.000	0.00	1.69	0.00	0.00	0.48	5.0	13.6	7.1	3.44	4.73	4.73	12	1.51	150.60	152.00	152.16	152.80	154.00	155.00	MH 24A-MH 24
4	3	45.000	0.13	1.69	0.26	0.03	0.48	5.0	13.5	7.2	3.46	12.20	4.79	12	10.00	152.00	156.50	153.16	157.29	155.00	165.00	AD 25-MH 24A
5	4	141.000	0.07	1.56	0.24	0.02	0.45	5.0	13.2	7.2	3.25	12.16	9.05	12	9.93	162.00	176.00	162.35	176.77	165.00	184.30	AD 26-AD 27
6	5	33.000	0.74	1.49	0.29	0.21	0.43	10.0	13.1	7.3	3.14	5.62	6.13	12	2.12	180.50	181.20	181.03	181.96	184.30	185.60	AD 27-AD 26
7	6	114.000	0.75	0.75	0.29	0.22	0.22	12.5	12.5	7.5	1.62	4.12	3.14	12	1.14	181.20	182.50	181.96	183.04	185.60	186.20	AD 28-AD 27
8	1	126.000	0.00	0.00	0.00	0.00	0.00	5.0	5.1	0.0	13.50	14.33	8.32	18	1.59	150.50	152.50	151.74	153.87	154.00	159.00	MH 3-MH 2
9	8	34.000	0.00	0.00	0.00	0.00	0.00	5.0	5.0	0.0	13.50	13.80	7.99	18	1.47	152.50	153.00	153.87	154.37	159.00	158.42	OCS 110-MH 3

Project File: Outlet 110 - 100 year.stm

Number of lines: 9

Run Date: 10/25/2020

NOTES:Intensity = 53.20 / (Inlet time + 3.70) ^ 0.71; Return period =Yrs. 100 ; c = cir e = ellip b = box

Hydraulic Grade Line Computations

Line	Size	Q	Downstream								Len	Upstream								Check		JL coeff	Minor loss
			Invert elev	HGL elev	Depth	Area	Vel	Vel head	EGL elev	Sf		Invert elev	HGL elev	Depth	Area	Vel	Vel head	EGL elev	Sf	Ave Sf	Enrgy loss		
	(in)	(cfs)	(ft)	(ft)	(ft)	(sqft)	(ft/s)	(ft)	(ft)	(%)	(ft)	(ft)	(ft)	(ft)	(sqft)	(ft/s)	(ft)	(ft)	(%)	(%)	(ft)	(K)	(ft)
1	18	16.88	150.00	151.50	1.50	1.74	9.55	1.42	152.92	2.203	11.000	150.30	151.74 j	1.44**	1.74	9.69	1.46	153.20	1.916	2.059	n/a	1.00	1.46
2	12	3.39	150.30	151.74	1.00	0.79	4.32	0.29	152.03	0.773	20.000	150.60	151.89	1.00	0.79	4.32	0.29	152.18	0.773	0.773	0.155	0.91	0.26
3	12	3.44	150.60	152.16	1.00	0.79	4.38	0.30	152.45	0.794	93.000	152.00	152.80	0.80	0.68	5.08	0.40	153.20	0.822	0.808	0.751	0.89	0.36
4	12	3.46	152.00	153.16	1.00	0.67	4.40	0.30	153.46	0.804	45.000	156.50	157.29 j	0.79**	0.67	5.17	0.42	157.71	0.853	0.829	n/a	0.63	n/a
5	12	3.25	162.00	162.35	0.35*	0.25	13.10	0.39	162.74	0.000	141.000	176.00	176.77	0.77**	0.65	5.00	0.39	177.16	0.000	0.000	n/a	1.45	0.56
6	12	3.14	180.50	181.03	0.53*	0.43	7.35	0.38	181.41	0.000	33.000	181.20	181.96	0.76**	0.64	4.91	0.38	182.33	0.000	0.000	n/a	0.50	n/a
7	12	1.62	181.20	181.96	0.76	0.43	2.54	0.22	182.18	0.000	114.000	182.50	183.04 j	0.54**	0.43	3.75	0.22	183.26	0.000	0.000	n/a	1.00	n/a
8	18	13.50	150.50	151.74	1.24	1.56	8.66	0.99	152.73	0.000	126.000	152.50	153.87	1.37**	1.69	7.99	0.99	154.86	0.000	0.000	n/a	0.30	n/a
9	18	13.50	152.50	153.87	1.37*	1.69	7.99	0.99	154.86	0.000	34.000	153.00	154.37	1.37**	1.69	7.99	0.99	155.36	0.000	0.000	n/a	1.00	n/a

Project File: Outlet 110 - 100 year.stm | Number of lines: 9 | Run Date: 10/25/2020

Notes: * depth assumed; ** Critical depth.; j-Line contains hyd. jump ; c = cir e = ellip b = box

Outlet Protection Calculations

Project:	Slate Upper School	By:	AWG	Date:	10/24/20
Location:	5100 Ridge Road, North Haven, CT	Checked:	FAB	Date:	10/25/20
Outlet I.D.	**FES 4**				

*Based on Connecticut DOT Drainage Manual, Section 11.13

Description:

FES 4

Design Criteria (25-yr Storm Event):

Q (cfs) =	6.47	R_p (ft)=	1.5
D (in) =	18	S_p (ft) =	1.5
V (fps) =	4	Tw (ft)=	1.5

Q= Flow rate at discharge point in cubic feet per second (cfs)
D= Outlet pipe diameter (in)
V= Flow velocity at discharge point (ft/s)
R_p= Maximum inside pipe rise (ft)
S_p= inside diametere for circular sections of maximum inside pipe span for non-circular sections (ft)
T_w= Tailwater depth (ft)

Based on **Table 11-13.1** use Type 'B' ---> TW≥ 0.5 Rp

Rip Rap Stone Size:

Velocity	Rip Rap Specification	D_{50} Stone Size
0-8 fps	Modified	5 inches

Preformed Scour Hole Dimensions:

F(ft)=0.5(R_p)	=	n/a
C(ft)=3.0(S_p)+6.0(F)	=	n/a
B(ft)=2.0(S_p)+6.0(F)	=	n/a

Rip Rap Splash Pad Dimensions:

L_a	=	14	ft
W1 = 3.0(S_p) min.	=	5	ft
W2 = 3.0(Sp)+0.4(La) min.	=	10	ft
d (Depth of Stone)	=	12	inches

Level Spreader Design

Level Spreader 110

Broad Crest Elevation (ft)	151.00	
Length (ft)	**35**	
Discharge Coefficient	3.2	
Elevation Increment	0.05	
Q-100 year (cfs)	16.88	(Outlet Pipe System)

Elevation (Feet)	Weir Discharge (cfs)	Area (sf)	Velocity (fps)
151.00	0.00	0.00	0.00
151.05	1.25	1.75	0.72
151.10	3.54	3.50	1.01
151.15	6.51	5.25	1.24
151.20	10.02	7.00	1.43
151.25	14.00	8.75	1.60
151.28	16.88	9.91	1.70
151.30	18.40	10.50	1.75
151.35	23.19	12.25	1.89
151.40	28.33	14.00	2.02
151.45	33.81	15.75	2.15
151.50	39.60	17.50	2.26

0' 15' 30'

0 1/2" 1"

MILONE & MACBROOM

99 REALTY DRIVE
CHESHIRE, CT 06410
203.271.1773
WWW.MMINC.COM

DESCRIPTION	DATE	BY

DRAINAGE AREA MAP - STORM DRAINAGE SYSTEM

SLATE UPPER SCHOOL

5100 RIDGE ROAD
NORTH HAVEN, CONNECTICUT

AWG	AWG	FAB
DESIGNED	DRAWN	CHECKED

SCALE: 1"=30'

DATE: OCTOBER 27, 2020

PROJECT NO.: 6156-03

SHEET NAME: **CB**

ATTACHMENT E

WATER QUALITY COMPUTATIONS

STORMWATER QUALITY CALCULATIONS
Water Quality Volume (WQV)

Basin ID	Total Area (ac.)	Impervious Area (ac.)	Percent Impervious	Volumetric Runoff Coeff., R	WQV (ac-ft)	Total Volume Required (ac-ft)	Total Volume Provided[1.] (ac-ft)
110	2.69	1.12	41.6%	0.42	0.095	0.095	***0.098***

1. - Volume provided at the lowest free-flowing hydraulic opening/structure in the outlet control structure

WQV = $\frac{(1.0 \text{ inches}) \times A \times R}{12}$

Where:

WQV = Water Quality Volume in acre-feet
A = Contributing Area in acres
R = 0.05 + 0.009 (I)
I = Site Imperviousness as percent

STORMWATER QUALITY CALCULATIONS
Water Quality Volume (WQV)

Stormwater Basin 110:

Elevation	Surface Area	Volume	Volume (ac-ft)	Cumulative Volume (ac-ft)
154.0	2,000	0.0	0.000	**0.000**
155.0	2,600	2,300	0.053	**0.053**
155.7	3,025	1,969	0.045	**0.098**

	MILONE AND MACBROOM, INC.		Project	**6156-03**
	COMPUTATION SHEET - WATER QUALITY FLOW (WQF)		Made By:	FAB
Subject:	**Slate Upper School - 5100 Ridge Road**		Date:	10/27/2020
	North Haven, CT		Chkd by:	
			Date:	

Contech CDS Unit (WS 11)

Contributing Basins			Imperv. Area (acres)	Total Area (acres)				
Total			1.12	2.69				
Table 4.1: WQV = $(P)(R_v)(A)/12$ =				0.095	acre-feet			
Where:								
I = % of Impervious Cover =				42%				
R_v = volumetric runoff coeff. 0.05 + 0.009(I) =				0.425				
P = design precipitation (1.0" for water quality storm) =					1	inch		
A = site area (acres) =				2.69	acres =	0.0042	$miles^2$	
Q = runoff depth (in watershed inches) = [WQV(acrefeet)]*[12(inches/foot)]/drainage area (acres)								
			Q =	0.425				
CN = $1000 / [10+ 5P + 10Q -10(Q^2 + 1.25QP)^{0.5}]$ =					92			
Where:								
Q = runoff depth (in watershed inches)								
			t_c =	0.189	hours			
Type III Rainfall Distribution:								
From Table 4-1, Ia =		0.174		Ia/P =	0.174			
(TR-55)								
From Exhibit 4-III, q_u =		540	csm/in.					
(TR-55)								
WQF = (qu)(A)(Q) =		0.96	cfs	**Contech CDS 2015-4-C Flow Rate = 1.4 cfs -> OK**				

2. Compute the time of concentration (t_c) based on the methods described in Chapter 3 of TR-55. A minimum value of 0.167 hours (10 minutes) should be used. For sheet flow, the flow path should not be longer than 300 feet.

3. Using the computed CN, t_c, and drainage area (A) in acres, compute the peak discharge for the water quality storm (i.e., the water quality flow [WQF]), based on the procedures described in Chapter 4 of TR-55.

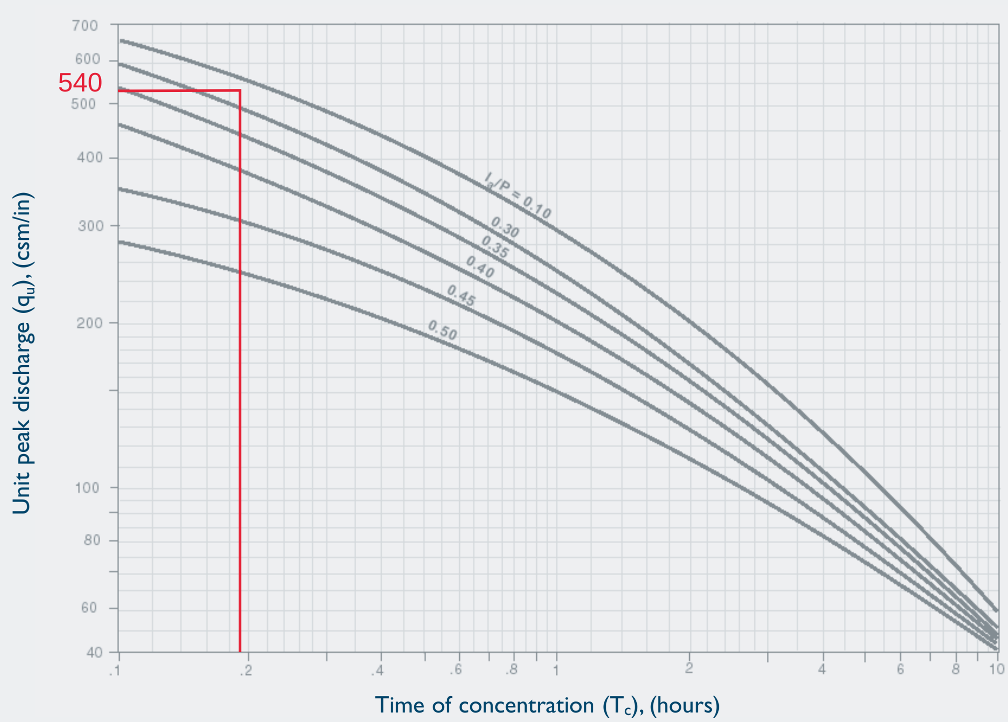
   ❍ *Read initial abstraction (I_a) from Table 4-1 in Chapter 4 of TR-55 (reproduced below); compute I_a/P*

Table 4-1 I_a values for runoff curve numbers

Curve number	I_a (in)	Curve number	I_a (in)	Curve number	I_a (in)	Curve number	I_a (in)
40	3.000	55	1.636	70	0.857	85	0.353
41	2.878	56	1.571	71	0.817	86	0.326
42	2.762	57	1.509	72	0.778	87	0.299
43	2.651	58	1.448	73	0.740	88	0.273
44	2.545	59	1.390	74	0.703	89	0.247
45	2.444	60	1.333	75	0.667	90	0.222
46	2.348	61	1.279	76	0.632	91	0.198
47	2.255	62	1.226	77	0.597	92	0.174
48	2.167	63	1.175	78	0.564	93	0.151
49	2.082	64	1.125	79	0.532	94	0.128
50	2.000	65	1.077	80	0.500	95	0.105
51	1.922	66	1.030	81	0.469	96	0.083
52	1.846	67	0.985	82	0.439	97	0.062
53	1.774	68	0.941	83	0.410	98	0.041
54	1.704	69	0.899	84	0.381		

   ❍ *Read the unit peak discharge (q_u) from Exhibit 4-III in Chapter 4 of TR-55 (reproduced below) for appropriate t_c*

Exhibit 4-111 Unit peak discharge (q_u) for NRCS (SCS) type III rainfall distribution



Product Flow Rates

CASCADE

Model	Treatment Rate (cfs)	Sediment Capacity[1] (CF)
CS-4	2.00	19
CS-5	3.50	29
CS-6	5.60	42
CS-8	12.00	75
CS-10	18.00	118

CDS

Model	Treatment Rate[2] (cfs)	Sediment Capacity[1] (CF)
1515-3	1.00	14
2015-4	1.40	25
2015-5	1.40	39
2015-6	1.40	57
2020-5	2.20	39
2020-6	2.20	57
2025-5	3.20	39
2025-6	3.20	57
3020-6	3.90	57
3025-6	5.00	57
3030-6	5.70	57
3035-6	6.50	57
4030-8	7.50	151
4040-8	9.50	151

VORTECHS

Model	Treatment Rate (cfs)	Sediment Capacity[3] (CF)
1000	1.60	16
2000	2.80	32
3000	4.50	49
4000	6.00	65
5000	8.50	86
7000	11.00	108
9000	14.00	130
11000	17.5	151
16000	25	192

STORMCEPTOR STC

Model	Treatment Rate (cfs)	Sediment Capacity[1] (CF)
STC 450i	0.40	46
STC 900	0.89	89
STC 2400	1.58	205
STC 4800	2.47	543
STC 7200	3.56	839
STC 11000	4.94	1086
STC 16000	7.12	1677

1 *Additional sediment storage capacity available – Check with your local representative for information.*

2 *Treatment Capacity is based on laboratory testing using OK-110 (average D50 particle size of approximately 100 microns) and a 2400 micron screen.*

3 *Maintenance recommended when sediment depth has accumulated to within 12-18 inches of the dry weather water surface elevation.*

STORMWATER SOLUTIONS

PIPE SOLUTIONS

STRUCTURES SOLUTIONS

CONTECH ENGINEERED SOLUTIONS

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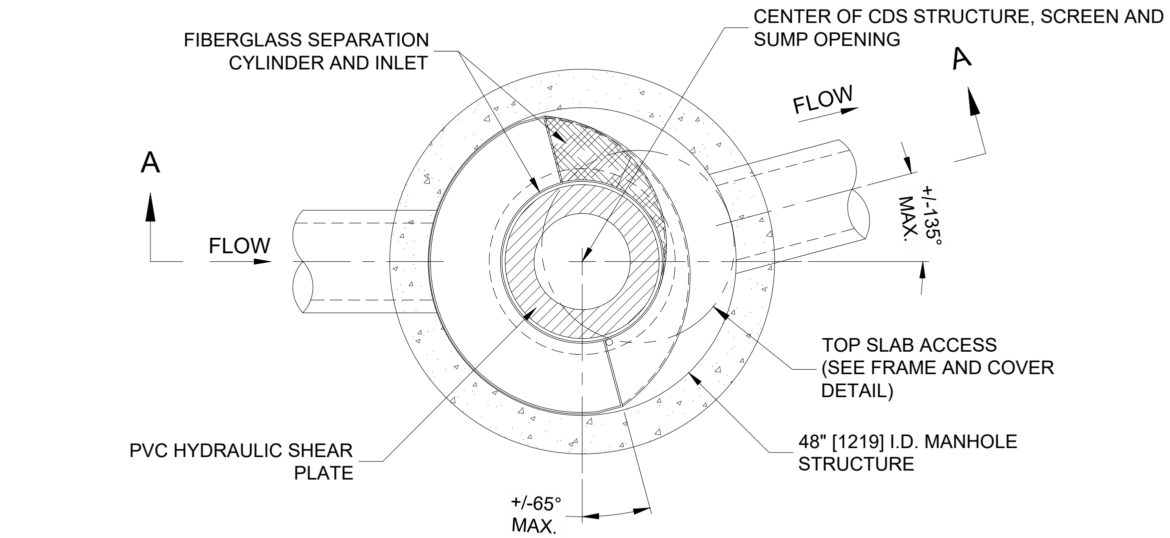
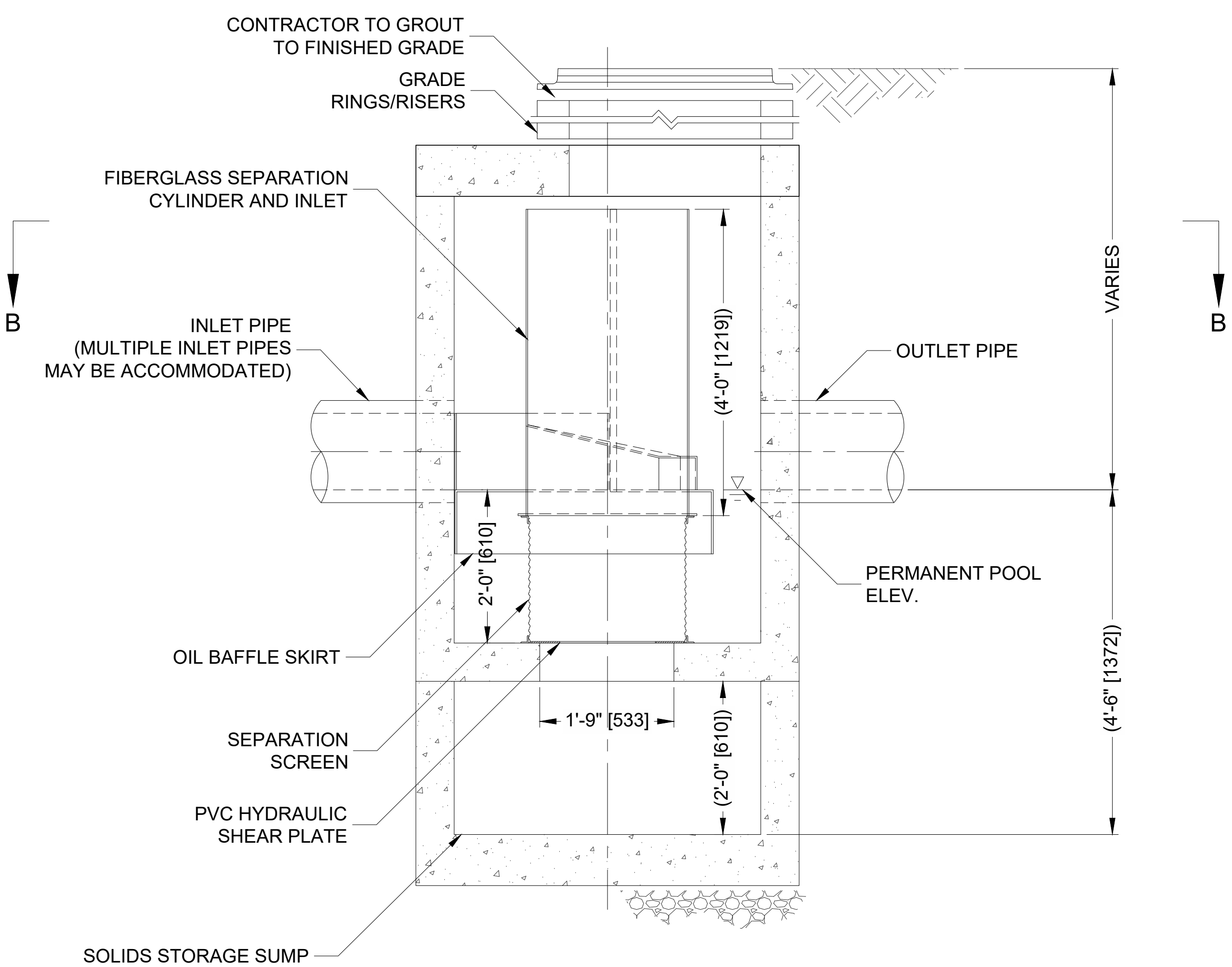


PLAN VIEW B-B

N.T.S.



ELEVATION A-A

N.T.S.

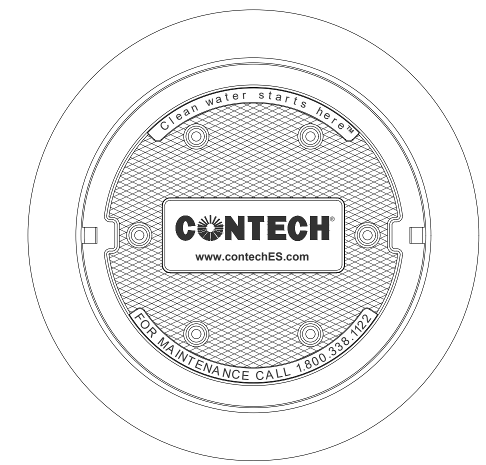
THIS PRODUCT MAY BE PROTECTED BY ONE OR MORE OF THE FOLLOWING U.S. PATENTS: 5,788,848; 6,641,720; 6,511,595; 6,581,783; RELATED FOREIGN PATENTS, OR OTHER PATENTS PENDING.

CDS2015-4-C DESIGN NOTES

CDS2015-4-C RATED TREATMENT CAPACITY IS 1.4 CFS, OR PER LOCAL REGULATIONS. MAXIMUM HYDRAULIC INTERNAL BYPASS CAPACITY IS 10.0 CFS. IF THE SITE CONDITIONS EXCEED 10.0 CFS, AN UPSTREAM BYPASS STRUCTURE IS REQUIRED.

THE STANDARD CDS2015-4-C CONFIGURATION IS SHOWN. ALTERNATE CONFIGURATIONS ARE AVAILABLE AND ARE LISTED BELOW. SOME CONFIGURATIONS MAY BE COMBINED TO SUIT SITE REQUIREMENTS.

CONFIGURATION DESCRIPTION
GRATED INLET ONLY (NO INLET PIPE)
GRATED INLET WITH INLET PIPE OR PIPES
CURB INLET ONLY (NO INLET PIPE)
CURB INLET WITH INLET PIPE OR PIPES



FRAME AND COVER

(DIAMETER VARIES)

N.T.S.

SITE SPECIFIC DATA REQUIREMENTS

STRUCTURE ID			
WATER QUALITY FLOW RATE (CFS OR L/s)			*
PEAK FLOW RATE (CFS OR L/s)			*
RETURN PERIOD OF PEAK FLOW (YRS)			*
SCREEN APERTURE (2400 OR 4700)			*
PIPE DATA:	I.E.	MATERIAL	DIAMETER
INLET PIPE 1	*	*	*
INLET PIPE 2	*	*	*
OUTLET PIPE	*	*	*
RIM ELEVATION			*
ANTI-FLOTATION BALLAST		WIDTH	HEIGHT
		*	*
NOTES/SPECIAL REQUIREMENTS:			

* PER ENGINEER OF RECORD

GENERAL NOTES

1. CONTECH TO PROVIDE ALL MATERIALS UNLESS NOTED OTHERWISE.
2. FOR SITE SPECIFIC DRAWINGS WITH DETAILED STRUCTURE DIMENSIONS AND WEIGHT, PLEASE CONTACT YOUR CONTECH ENGINEERED SOLUTIONS LLC REPRESENTATIVE. www.ContechES.com
3. CDS WATER QUALITY STRUCTURE SHALL BE IN ACCORDANCE WITH ALL DESIGN DATA AND INFORMATION CONTAINED IN THIS DRAWING. CONTRACTOR TO CONFIRM STRUCTURE MEETS REQUIREMENTS OF PROJECT.
4. STRUCTURE SHALL MEET AASHTO HS20 LOAD RATING, ASSUMING EARTH COVER OF 0' - 2', AND GROUNDWATER ELEVATION AT, OR BELOW, THE OUTLET PIPE INVERT ELEVATION. ENGINEER OF RECORD TO CONFIRM ACTUAL GROUNDWATER ELEVATION. CASTINGS SHALL MEET AASHTO M306 AND BE CAST WITH THE CONTECH LOGO..
5. IF REQUIRED, PVC HYDRAULIC SHEAR PLATE IS PLACED ON SHELF AT BOTTOM OF SCREEN CYLINDER. REMOVE AND REPLACE AS NECESSARY DURING MAINTENANCE CLEANING.
6. CDS STRUCTURE SHALL BE PRECAST CONCRETE CONFORMING TO ASTM C-478 AND AASHTO LOAD FACTOR DESIGN METHOD.

INSTALLATION NOTES

A. ANY SUB-BASE, BACKFILL DEPTH, AND/OR ANTI-FLOTATION PROVISIONS ARE SITE-SPECIFIC DESIGN CONSIDERATIONS AND SHALL BE SPECIFIED BY ENGINEER OF RECORD.
B. CONTRACTOR TO PROVIDE EQUIPMENT WITH SUFFICIENT LIFTING AND REACH CAPACITY TO LIFT AND SET THE CDS MANHOLE STRUCTURE.
C. CONTRACTOR TO INSTALL JOINT SEALANT BETWEEN ALL STRUCTURE SECTIONS AND ASSEMBLE STRUCTURE.
D. CONTRACTOR TO PROVIDE, INSTALL, AND GROUT INLET AND OUTLET PIPE(S). MATCH PIPE INVERTS WITH ELEVATIONS SHOWN. ALL PIPE CENTERLINES TO MATCH PIPE OPENING CENTERLINES.
E. CONTRACTOR TO TAKE APPROPRIATE MEASURES TO ASSURE UNIT IS WATER TIGHT, HOLDING WATER TO FLOWLINE INVERT MINIMUM. IT IS SUGGESTED THAT ALL JOINTS BELOW PIPE INVERTS ARE GROUTED.

CONTECH® ENGINEERED SOLUTIONS LLC
www.contechES.com
9025 Centre Pointe Dr., Suite 400, West Chester, OH 45069
800-338-1122 513-645-7000 513-645-7993 FAX

CDS2015-4-C ONLINE CDS STANDARD DETAIL

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CONTECH® ENGINEERED SOLUTIONS

CDS Guide
Operation, Design, Performance and Maintenance

CDS®

Using patented continuous deflective separation technology, the CDS system screens, separates and traps debris, sediment, and oil and grease from stormwater runoff. The indirect screening capability of the system allows for 100% removal of floatables and neutrally buoyant material without blinding. Flow and screening controls physically separate captured solids, and minimize the re-suspension and release of previously trapped pollutants. Inline units can treat up to 6 cfs, and internally bypass flows in excess of 50 cfs (1416 L/s). Available precast or cast-in-place, offline units can treat flows from 1 to 300 cfs (28.3 to 8495 L/s). The pollutant removal capacity of the CDS system has been proven in lab and field testing.

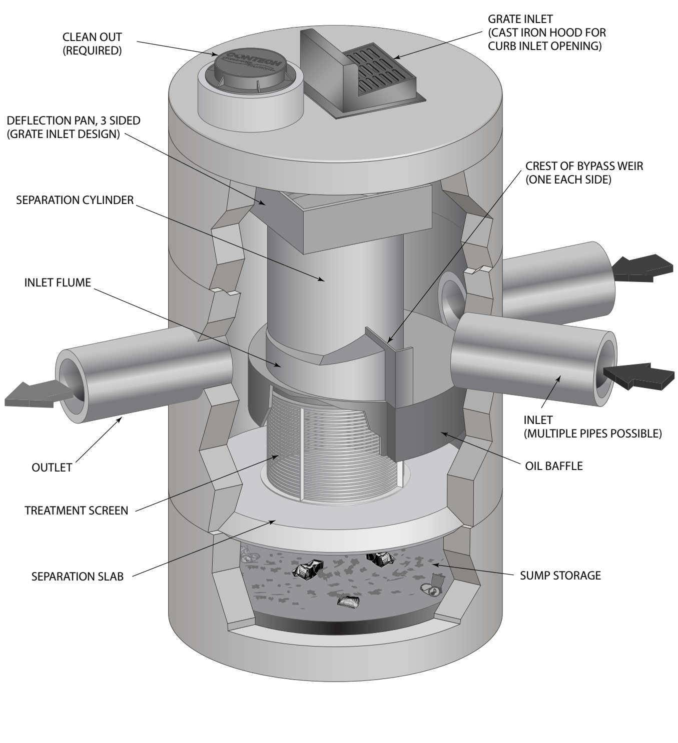
Operation Overview

Stormwater enters the diversion chamber where the diversion weir guides the flow into the unit's separation chamber and pollutants are removed from the flow. All flows up to the system's treatment design capacity enter the separation chamber and are treated.

Swirl concentration and screen deflection force floatables and solids to the center of the separation chamber where 100% of floatables and neutrally buoyant debris larger than the screen apertures are trapped.

Stormwater then moves through the separation screen, under the oil baffle and exits the system. The separation screen remains clog free due to continuous deflection.

During the flow events exceeding the treatment design capacity, the diversion weir bypasses excessive flows around the separation chamber, so captured pollutants are retained in the separation cylinder.



Design Basics

There are three primary methods of sizing a CDS system. The Water Quality Flow Rate Method determines which model size provides the desired removal efficiency at a given flow rate for a defined particle size. The Rational Rainfall Method™ or the and Probabilistic Method is used when a specific removal efficiency of the net annual sediment load is required.

Typically in the Unites States, CDS systems are designed to achieve an 80% annual solids load reduction based on lab generated performance curves for a gradation with an average particle size (d50) of 125 microns (μm). For some regulatory environments, CDS systems can also be designed to achieve an 80% annual solids load reduction based on an average particle size (d50) of 75 microns (μm) or 50 microns (μm).

Water Quality Flow Rate Method

In some cases, regulations require that a specific treatment rate, often referred to as the water quality design flow (WQQ), be treated. This WQQ represents the peak flow rate from either an event with a specific recurrence interval, e.g. the six-month storm, or a water quality depth, e.g. 1/2-inch (13 mm) of rainfall.

The CDS is designed to treat all flows up to the WQQ. At influent rates higher than the WQQ, the diversion weir will direct most flow exceeding the WQQ around the separation chamber. This allows removal efficiency to remain relatively constant in the separation chamber and eliminates the risk of washout during bypass flows regardless of influent flow rates.

Treatment flow rates are defined as the rate at which the CDS will remove a specific gradation of sediment at a specific removal efficiency. Therefore the treatment flow rate is variable, based on the gradation and removal efficiency specified by the design engineer.

Rational Rainfall Method™

Differences in local climate, topography and scale make every site hydraulically unique. It is important to take these factors into consideration when estimating the long-term performance of any stormwater treatment system. The Rational Rainfall Method combines site-specific information with laboratory generated performance data, and local historical precipitation records to estimate removal efficiencies as accurately as possible.

Short duration rain gauge records from across the United States and Canada were analyzed to determine the percent of the total annual rainfall that fell at a range of intensities. US stations' depths were totaled every 15 minutes, or hourly, and recorded in 0.01-inch increments. Depths were recorded hourly with 1-mm resolution at Canadian stations. One trend was consistent at all sites; the vast majority of precipitation fell at low intensities and high intensity storms contributed relatively little to the total annual depth.

These intensities, along with the total drainage area and runoff coefficient for each specific site, are translated into flow rates using the Rational Rainfall Method. Since most sites are relatively small and highly impervious, the Rational Rainfall Method is appropriate. Based on the runoff flow rates calculated for each intensity, operating rates within a proposed CDS system are determined. Performance efficiency curve determined from full scale laboratory tests on defined sediment PSDs is applied to calculate solids removal efficiency. The relative removal efficiency at each operating rate is added to produce a net annual pollutant removal efficiency estimate.

Probabilistic Rational Method

The Probabilistic Rational Method is a sizing program Contech developed to estimate a net annual sediment load reduction for a particular CDS model based on site size, site runoff coefficient, regional rainfall intensity distribution, and anticipated pollutant characteristics.

The Probabilistic Method is an extension of the Rational Method used to estimate peak discharge rates generated by storm events of varying statistical return frequencies (e.g. 2-year storm event). Under the Rational Method, an adjustment factor is used to adjust the runoff coefficient estimated for the 10-year event, correlating a known hydrologic parameter with the target storm event. The rainfall intensities vary depending on the return frequency of the storm event under consideration. In general, these two frequency dependent parameters (rainfall intensity and runoff coefficient) increase as the return frequency increases while the drainage area remains constant.

These intensities, along with the total drainage area and runoff coefficient for each specific site, are translated into flow rates using the Rational Method. Since most sites are relatively small and highly impervious, the Rational Method is appropriate. Based on the runoff flow rates calculated for each intensity, operating rates within a proposed CDS are determined. Performance efficiency curve on defined sediment PSDs is applied to calculate solids removal efficiency. The relative removal efficiency at each operating rate is added to produce a net annual pollutant removal efficiency estimate.

Treatment Flow Rate

The inlet throat area is sized to ensure that the WQQ passes through the separation chamber at a water surface elevation equal to the crest of the diversion weir. The diversion weir bypasses excessive flows around the separation chamber, thus preventing re-suspension or re-entrainment of previously captured particles.

Hydraulic Capacity

The hydraulic capacity of a CDS system is determined by the length and height of the diversion weir and by the maximum allowable head in the system. Typical configurations allow hydraulic capacities of up to ten times the treatment flow rate. The crest of the diversion weir may be lowered and the inlet throat may be widened to increase the capacity of the system at a given water surface elevation. The unit is designed to meet project specific hydraulic requirements.

Performance

Full-Scale Laboratory Test Results

A full-scale CDS system (Model CDS2020-5B) was tested at the facility of University of Florida, Gainesville, FL. This CDS unit was evaluated under controlled laboratory conditions of influent flow rate and addition of sediment.

Two different gradations of silica sand material (UF Sediment & OK-110) were used in the CDS performance evaluation. The particle size distributions (PSDs) of the test materials were analyzed using standard method "Gradation ASTM D-422 "Standard Test Method for Particle-Size Analysis of Soils" by a certified laboratory.

UF Sediment is a mixture of three different products produced by the U.S. Silica Company: "Sil-Co-Sil 106", "#1 DRY" and "20/40 Oil Frac". Particle size distribution analysis shows that the UF Sediment has a very fine gradation (d50 = 20 to 30 μm) covering a wide size range (Coefficient of Uniformity, C averaged at 10.6). In comparison with the hypothetical TSS gradation specified in the NJDEP (New Jersey Department of Environmental Protection) and NJCAT (New Jersey Corporation for Advanced Technology) protocol for lab testing, the UF Sediment covers a similar range of particle size but with a finer d50 (d50 for NJDEP is approximately 50 μm) (NJDEP, 2003).

The OK-110 silica sand is a commercial product of U.S. Silica Sand. The particle size distribution analysis of this material, also included in Figure 1, shows that 99.9% of the OK-110 sand is finer than 250 microns, with a mean particle size (d50) of 106 microns. The PSDs for the test material are shown in Figure 1.

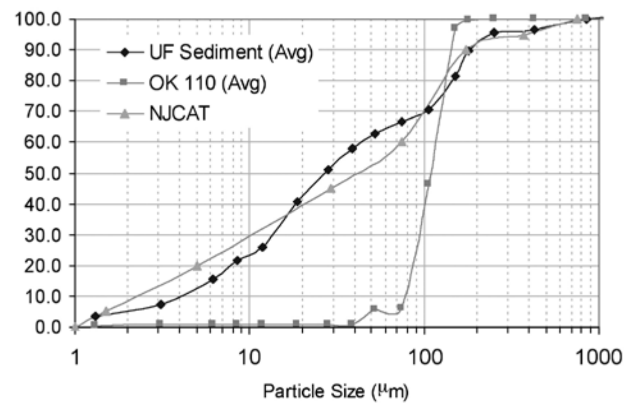


Figure 1. Particle size distributions

Tests were conducted to quantify the performance of a specific CDS unit (1.1 cfs (31.3-L/s) design capacity) at various flow rates, ranging from 1% up to 125% of the treatment design capacity of the unit, using the 2400 micron screen. All tests were conducted with controlled influent concentrations of approximately 200 mg/L. Effluent samples were taken at equal time intervals across the entire duration of each test run. These samples were then processed with a Dekaport Cone sample splitter to obtain representative sub-samples for Suspended Sediment Concentration (SSC) testing using ASTM D3977-97 "Standard Test Methods for Determining Sediment Concentration in Water Samples", and particle size distribution analysis.

Results and Modeling

Based on the data from the University of Florida, a performance model was developed for the CDS system. A regression analysis was used to develop a fitting curve representative of the scattered data points at various design flow rates. This model, which demonstrated good agreement with the laboratory data, can then be used to predict CDS system performance with respect to SSC removal for any particle size gradation, assuming the particles are inorganic sandy-silt. Figure 2 shows CDS predictive performance for two typical particle size gradations (NJCAT gradation and OK-110 sand) as a function of operating rate.

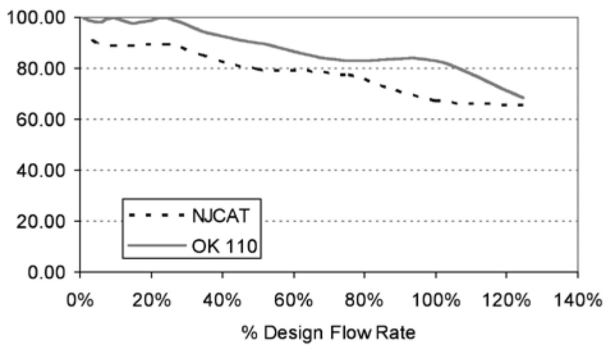


Figure 2. CDS stormwater treatment predictive performance for various particle gradations as a function of operating rate.

Many regulatory jurisdictions set a performance standard for hydrodynamic devices by stating that the devices shall be capable of achieving an 80% removal efficiency for particles having a mean particle size (d50) of 125 microns (e.g. Washington State Department of Ecology — WASDOE - 2008). The model can be used to calculate the expected performance of such a PSD (shown in Figure 3). The model indicates (Figure 4) that the CDS system with 2400 micron screen achieves approximately 80% removal at the design (100%) flow rate, for this particle size distribution (d50 = 125 μm).

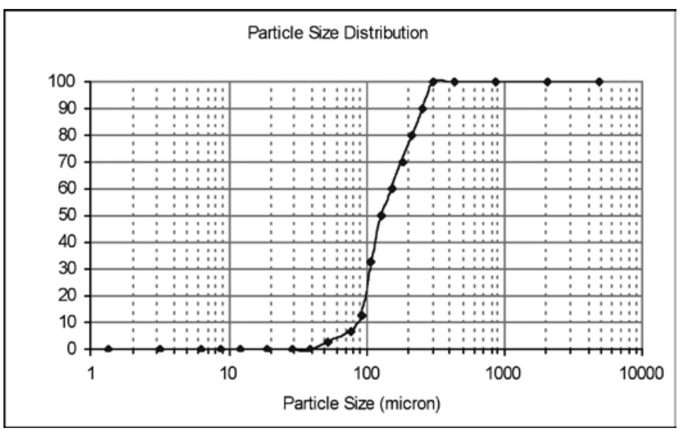


Figure 3. WASDOE PSD

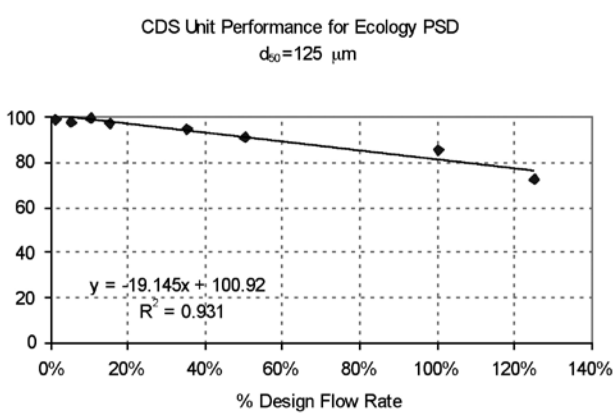


Figure 4. Modeled performance for WASDOE PSD.

Maintenance

The CDS system should be inspected at regular intervals and maintained when necessary to ensure optimum performance. The rate at which the system collects pollutants will depend more heavily on site activities than the size of the unit. For example, unstable soils or heavy winter sanding will cause the grit chamber to fill more quickly but regular sweeping of paved surfaces will slow accumulation.

Inspection

Inspection is the key to effective maintenance and is easily performed. Pollutant transport and deposition may vary from year to year and regular inspections will help ensure that the system is cleaned out at the appropriate time. At a minimum, inspections should be performed twice per year (e.g. spring and fall) however more frequent inspections may be necessary in climates where winter sanding operations may lead to rapid accumulations, or in equipment washdown areas. Installations should also be inspected more frequently where excessive amounts of trash are expected.

The visual inspection should ascertain that the system components are in working order and that there are no blockages or obstructions in the inlet and separation screen. The inspection should also quantify the accumulation of hydrocarbons, trash, and sediment in the system. Measuring pollutant accumulation can be done with a calibrated dipstick, tape measure or other measuring instrument. If absorbent material is used for enhanced removal of hydrocarbons, the level of discoloration of the sorbent material should also be identified

during inspection. It is useful and often required as part of an operating permit to keep a record of each inspection. A simple form for doing so is provided.

Access to the CDS unit is typically achieved through two manhole access covers. One opening allows for inspection and cleanout of the separation chamber (cylinder and screen) and isolated sump. The other allows for inspection and cleanout of sediment captured and retained outside the screen. For deep units, a single manhole access point would allows both sump cleanout and access outside the screen.

The CDS system should be cleaned when the level of sediment has reached 75% of capacity in the isolated sump or when an appreciable level of hydrocarbons and trash has accumulated. If absorbent material is used, it should be replaced when significant discoloration has occurred. Performance will not be impacted until 100% of the sump capacity is exceeded however it is recommended that the system be cleaned prior to that for easier removal of sediment. The level of sediment is easily determined by measuring from finished grade down to the top of the sediment pile. To avoid underestimating the level of sediment in the chamber, the measuring device must be lowered to the top of the sediment pile carefully. Particles at the top of the pile typically offer less resistance to the end of the rod than consolidated particles toward the bottom of the pile. Once this measurement is recorded, it should be compared to the as-built drawing for the unit to determine weather the height of the sediment pile off the bottom of the sump floor exceeds 75% of the total height of isolated sump.

Cleaning

Cleaning of a CDS systems should be done during dry weather conditions when no flow is entering the system. The use of a vacuum truck is generally the most effective and convenient method of removing pollutants from the system. Simply remove the manhole covers and insert the vacuum hose into the sump. The system should be completely drained down and the sump fully evacuated of sediment. The area outside the screen should also be cleaned out if pollutant build-up exists in this area.

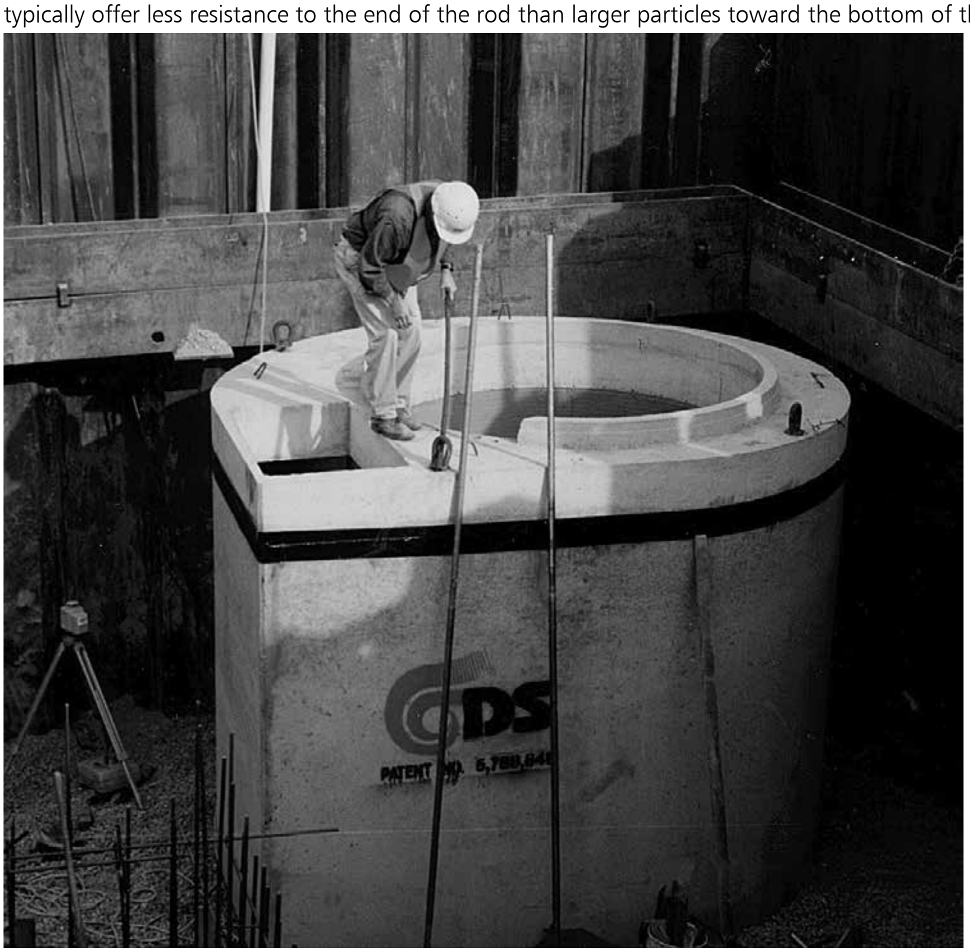
In installations where the risk of petroleum spills is small, liquid contaminants may not accumulate as quickly as sediment. However, the system should be cleaned out immediately in the event of an oil or gasoline spill. Motor oil and other hydrocarbons that accumulate on a more routine basis should be removed when an appreciable layer has been captured. To remove these pollutants, it may be preferable to use absorbent pads since they are usually less expensive to dispose than the oil/water emulsion that may be created by vacuuming the oily layer. Trash and debris can be netted out to separate it from the other pollutants. The screen should be cleaned to ensure it is free of trash and debris.

Manhole covers should be securely seated following cleaning activities to prevent leakage of runoff into the system from above and also to ensure that proper safety precautions have been followed. Confined space entry procedures need to be followed if physical access is required. Disposal of all material removed from the CDS system should be done in accordance with local regulations. In many jurisdictions, disposal of the sediments may be handled in the same manner as the disposal of sediments removed from catch basins or deep sump manholes. Check your local regulations for specific requirements on disposal.

CDS Model	Diameter		Distance from Water Surface to Top of Sediment Pile		Sediment Storage Capacity	
	ft	m	ft	m	y^3	m^3
CDS1515	3	0.9	3.0	0.9	0.5	0.4
CDS2015	4	1.2	3.0	0.9	0.9	0.7
CDS2015	5	1.5	3.0	0.9	1.3	1.0
CDS2020	5	1.5	3.5	1.1	1.3	1.0
CDS2025	5	1.5	4.0	1.2	1.3	1.0
CDS3020	6	1.8	4.0	1.2	2.1	1.6
CDS3025	6	1.8	4.0	1.2	2.1	1.6
CDS3030	6	1.8	4.6	1.4	2.1	1.6
CDS3035	6	1.8	5.0	1.5	2.1	1.6
CDS4030	8	2.4	4.6	1.4	5.6	4.3
CDS4040	8	2.4	5.7	1.7	5.6	4.3
CDS4045	8	2.4	6.2	1.9	5.6	4.3
CDS5640	10	3.0	6.3	1.9	8.7	6.7
CDS5653	10	3.0	7.7	2.3	8.7	6.7
CDS5668	10	3.0	9.3	2.8	8.7	6.7
CDS5678	10	3.0	10.3	3.1	8.7	6.7

Table 1: CDS Maintenance Indicators and Sediment Storage Capacities

Note: To avoid underestimating the volume of sediment in the chamber, carefully lower the measuring device to the top of the sediment pile. Finer silty particles at the top of the pile may be more difficult to feel with a measuring stick. These finer particles typically offer less resistance to the end of the rod than larger particles toward the bottom of the pile.



CDS Inspection & Maintenance Log

CDS Model: ____________________ Location: ____________________

Date	Water depth to sediment[1]	Floatable Layer Thickness[2]	Describe Maintenance Performed	Maintenance Personnel	Comments

1. The water depth to sediment is determined by taking two measurements with a stadia rod: one measurement from the manhole opening to the top of the sediment pile and the other from the manhole opening to the water surface. If the difference between these measurements is less than the values listed in table 1 the system should be cleaned out. **Note: to avoid underestimating the volume of sediment in the chamber, the measuring device must be carefully lowered to the top of the sediment pile**.

2. For optimum performance, the system should be cleaned out when the floating hydrocarbon layer accumulates to an appreciable thickness. In the event of an oil spill, the system should be cleaned immediately.

SUPPORT

- Drawings and specifications are available at www.ContechES.com.
- Site-specific design support is available from our engineers.

CONTECH ENGINEERED SOLUTIONS
800-338-1122
www.ContechES.com

cds_manual 3/17 PDF

ATTACHMENT F

HYDROLOGIC ANALYSIS – INPUT COMPUTATIONS

Curve Number Calculations

Project: Slate Upper School
Location: 5100 Ridge Road
North Haven, Connecticut
By: FAB Date: 10/27/20 Checked: ____ Date: ____
Circle one: ***Present*** Developed Watershed: EX WS10

Soil Name and Hydrologic Group (appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN Value [1.]			Area (Acres / Sq. Ft. / %)	Product of CN x Area
		Table 2-2	Figure 2-3	Figure 2-4		
B Soil	Woods - Good Condition	55			0.72	39.41
B Soil	Open Space - Good Condition	61			0.43	26.02
C Soil	Woods - Good Condition	70			1.29	90.17
C Soil	Open Space - Good Condition	74			1.67	123.73
C Soil	Gravel	89			0.01	0.65
D Soil	Woods - Good Condition	77			0.04	2.86
D Soil	Open Space - Good Condition	80			0.01	0.98
N/A	Existing Building	98			0.13	12.56
N/A	Existing Paved/Impervious	98			0.49	47.80
				Totals =	4.78	344.17

(0.00746 sq mi)

CN (weighted) = $\frac{\text{total product}}{\text{total area}} = \frac{344.17}{4.78}$ Use CN = **72**

Curve Number Calculations

Project: Slate Upper School
Location: 5100 Ridge Road
North Haven, Connecticut
By: FAB Date: 10/27/20 Checked: ______ Date: ______
Circle one: Present ***Developed*** Watershed: PR WS10

Soil Name and Hydrologic Group (appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN Value [1.]			Area (Acres) Sq. Ft. %	Product of CN x Area
		Table 2-2	Figure 2-3	Figure 2-4		
B Soil	Woods - Good Condition	55			0.22	11.86
B Soil	Open Space - Good Condition	61			0.20	12.25
C Soil	Woods - Good Condition	70			0.89	62.60
C Soil	Open Space - Good Condition	74			0.55	40.76
D Soil	Woods - Good Condition	77			0.02	1.34
D Soil	Open Space - Good Condition	80			0.03	2.56
N/A	Existing Building	98			0.07	6.64
N/A	Existing Paved/Impervious	98			0.11	10.77
					Totals = 2.09	148.78

(0.00326 sq mi)

$$\text{CN (weighted)} = \frac{\text{total product}}{\text{total area}} = \frac{148.78}{2.09}$$

Use CN = **71**

Curve Number Calculations

Project: Slate Upper School
Location: 5100 Ridge Road
North Haven, Connecticut
By: FAB Date: 10/27/20 Checked: ______ Date: ______
Circle one: Present ***Developed*** Watershed: PR WS11

Soil Name and Hydrologic Group (appendix A)	Cover Description (cover type, treatment, and hydrologic condition; percent impervious; unconnected/connected impervious area ratio)	CN Value [1.]			Area (Acres / Sq. Ft. / %)	Product of CN x Area
		Table 2-2	Figure 2-3	Figure 2-4		
B Soil	Woods - Good Condition	55			0.42	23.36
B Soil	Open Space - Good Condition	61			0.20	12.14
C Soil	Woods - Good Condition	70			0.11	7.43
C Soil	Open Space - Good Condition	74			0.84	62.00
N/A	Proposed Building	98			0.24	23.48
N/A	Existing Paved/Impervious	98			0.04	4.26
N/A	Proposed Paved/Impervious	98			0.84	81.96
				Totals =	2.69	214.64

(0.00420 sq mi)

CN (weighted) = $\frac{\text{total product}}{\text{total area}}$ = $\frac{214.64}{2.69}$ Use CN = **80**

Time of Concentration (T_c) or Travel Time (T_t) Worksheet

Project: Slate Upper School By: FAB Date: 10/27/20
Location: 5100 Ridge Road, North Haven, CT Checked: Date:
Circle one: ***Present*** Developed Watershed: EXWS 10
Circle one: ***T_c*** T_t Subwatershed:

Sheet flow (applicable to T_c only)

	Unit	Segment ID A-B		Total
1. Surface description (Table 3-1)		WOODS		
2. Manning's roughness coeff. for sheet flow, n (Table 3-1)		0.400		
3. Flow Length, L (< 300ft)	ft.	70.0		
4. Two-year 24-hr rainfall, P_2	in.	3.50		
5. Land slope, s	ft./ft.	0.045		
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}(s^{0.4})}$	hr.	0.186	=	0.186

Shallow concentrated flow (assume hyd. radius = depth of flow)

	Unit	B-C		C-D		D-E		E-F		Total
7. Surface description		WOODS		BIT		WOODS		GRASS		
8. Manning's roughness coeff., n		0.100		0.015		0.100		0.080		
9. Paved or unpaved		UNPVD		PVD		UNPVD		UNPVD		
10. Depth of flow, d (default values: d=.4 unpaved, d=.2 paved)	ft.	0.40		0.20		0.40		0.40		
11. Flow Length, L	ft.	210.0		101.0		55.0		330.0		
12. Watercourse slope, s	ft./ft.	0.15		0.16		0.16		0.11		
13. Average velocity, $V = \frac{1.49}{n}(d^{2/3})(s^{1/2})$	fps.	3.13		13.59		3.24		3.37		
14. $T_t = \frac{L}{3600 * V}$	hr.	0.019	+	0.002	+	0.005	+	0.027	=	0.053

Channel flow

	Unit	Segment ID								Total
15. Channel Bottom width, b	ft.									
16. Horizontal side slope component, z (z horiz:1 vert)	ft.									
17. Depth of flow, d	ft.									
18. Cross sectional flow area, A (assume trapazoidal)	$ft.^2$									
19. Wetted perimeter, P_w	ft.									
20. Hydraulic Radius, $R = \frac{A}{P_w}$	ft.									
21. Channel slope, s	ft./ft.									
22. Manning's roughness coeff., n										
23. $V = \frac{1.49}{n}(R^{2/3})(s^{1/2})$	fps.									
24. Flow length, L	ft.									
25. $T_t = \frac{L}{3600 * V}$	hr.						+		=	0.000

26. Watershed or subarea T_c or T_t (add T_t in steps 6, 14 & 25) hr. 0.239

Time of Concentration (T_c) or Travel Time (T_t) Worksheet

Project: Slate Upper School By: FAB Date: 10/27/20
Location: 5100 Ridge Road, North Haven, CT Checked: Date:
Circle one: Present ***Developed*** Watershed: PRWS 10
Circle one: ***T_c*** T_t Subwatershed:

Sheet flow (applicable to T_c only)

	Unit	A-B		Total
Segment ID		A-B		
1. Surface description (Table 3-1)		WOODS		
2. Manning's roughness coeff. for sheet flow, n (Table 3-1)		0.400		
3. Flow Length, L (< 300ft)	ft.	70.0		
4. Two-year 24-hr rainfall, P_2	in.	3.50		
5. Land slope, s	ft./ft.	0.045		
6. $T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}(s^{0.4})}$	hr.	0.186	=	0.186

Shallow concentrated flow (assume hyd. radius = depth of flow)

	Unit	B-C		C-D		D-E		E-F		Total
Segment ID		B-C		C-D		D-E		E-F		
7. Surface description		WOODS		BIT		WOODS		GRASS		
8. Manning's roughness coeff., n		0.100		0.015		0.100		0.080		
9. Paved or unpaved		UNPVD		PVD		UNPVD		UNPVD		
10. Depth of flow, d (default values: d=.4 unpaved, d=.2 paved)	ft.	0.40		0.20		0.40		0.40		
11. Flow Length, L	ft.	210.0		101.0		25.0		10.0		
12. Watercourse slope, s	ft./ft.	0.15		0.16		0.16		0.30		
13. Average velocity, $V = \frac{1.49}{n}(d^{2/3})(s^{1/2})$	fps.	3.13		13.59		3.24		5.54		
14. $T_t = \frac{L}{3600 * V}$	hr.	0.019	+	0.002	+	0.002	+	0.001	=	0.023

Channel flow

	Unit	F-G								Total
Segment ID		F-G								
15. Channel Bottom width, b	ft.	12" HDPE								
16. Horizontal side slope component, z (z horiz:1 vert)	ft.	--								
17. Depth of flow, d	ft.	FULL								
18. Cross sectional flow area, A (assume trapazoidal)	$ft.^2$	0.79								
19. Wetted perimeter, P_w	ft.	3.14								
20. Hydraulic Radius, $R = \frac{A}{P_w}$	ft.	0.25								
21. Channel slope, s	ft./ft.	0.07								
22. Manning's roughness coeff., n		0.012								
23. $V = \frac{1.49}{n}(R^{2/3})(s^{1/2})$	fps.	13.09								
24. Flow length, L	ft.	465.0								
25. $T_t = \frac{L}{3600 * V}$	hr.	0.010					+		=	0.010

26. Watershed or subarea T_c or T_t (add T_t in steps 6, 14 & 25) hr. 0.219

Time of Concentration (T_c) or Travel Time (T_t) Worksheet

Project: Slate Upper School — By: FAB — Date: 10/27/20
Location: 5100 Ridge Road, North Haven, CT — Checked: — Date:
Circle one: Present **Developed** — Watershed: PRWS 11
Circle one: **T_c** T_t — Subwatershed:

Sheet flow (applicable to T_c only)

		Segment ID	A-B	
1.	Surface description (Table 3-1)		WOODS	
2.	Manning's roughness coeff. for sheet flow, n (Table 3-1)		0.400	
3.	Flow Length, L (< 300ft)	ft.	65.0	
4.	Two-year 24-hr rainfall, P_2	in.	3.50	
5.	Land slope, s	ft./ft.	0.045	
6.	$T_t = \frac{0.007(nL)^{0.8}}{P_2^{0.5}(s^{0.4})}$	hr.	0.175	= 0.175

Shallow concentrated flow (assume hyd. radius = depth of flow)

		Segment ID	B-C	C-D	D-E	E-F	F-G
7.	Surface description		WOODS	BIT	WOODS	GRASS	BIT
8.	Manning's roughness coeff., n		0.100	0.015	0.100	0.080	0.015
9.	Paved or unpaved		UNPVD	PVD	UNPVD	UNPVD	PVD
10.	Depth of flow, d (default values: d=.4 unpaved, d=.2 paved)	ft.	0.40	0.20	0.40	0.40	0.20
11.	Flow Length, L	ft.	270.0	38.0	25.0	40.0	100.0
12.	Watercourse slope, s	ft./ft.	0.17	0.16	0.16	0.20	0.03
13.	Average velocity, $V = \frac{1.49}{n}(d^{2/3})(s^{1/2})$	fps.	3.34	13.59	3.24	4.52	5.88
14.	$T_t = \frac{L}{3600 * V}$	hr.	0.022	+ 0.001	+ 0.002	+ 0.002	+ 0.005
							= 0.033

Channel flow

		Segment ID	G-H				
15.	Channel Bottom width, b	ft.	15" HDPE				
16.	Horizontal side slope component, z (z horiz:1 vert)	ft.	--				
17.	Depth of flow, d	ft.	FULL				
18.	Cross sectional flow area, A (assume trapazoidal)	$ft.^2$	1.23				
19.	Wetted perimeter, P_w	ft.	3.93				
20.	Hydraulic Radius, $R = \frac{A}{P_w}$	ft.	0.31				
21.	Channel slope, s	ft./ft.	0.05				
22.	Manning's roughness coeff., n		0.012				
23.	$V = \frac{1.49}{n}(R^{2/3})(s^{1/2})$	fps.	12.41				
24.	Flow length, L	ft.	380.0				
25.	$T_t = \frac{L}{3600 * V}$	hr.	0.009			+	= 0.009
26.	Watershed or subarea T_c or T_t (add T_t in steps 6, 14 & 25)	hr.					0.216

NOAA Atlas 14, Volume 10, Version 3
Location name: North Haven, Connecticut, USA*
Latitude: 41.4214°, Longitude: -72.8826°
Elevation: 181.97 ft**
* source: ESRI Maps
** source: USGS

POINT PRECIPITATION FREQUENCY ESTIMATES

Sanja Perica, Sandra Pavlovic, Michael St. Laurent, Carl Trypaluk, Dale Unruh, Orlan Wilhite

NOAA, National Weather Service, Silver Spring, Maryland

PF_tabular | PF_graphical | Maps_&_aerials

PF tabular

PDS-based point precipitation frequency estimates with 90% confidence intervals (in inches)[1]

Duration	Average recurrence interval (years)									
	1	2	5	10	25	50	100	200	500	1000
5-min	0.341 (0.262-0.429)	0.413 (0.317-0.520)	0.531 (0.405-0.671)	0.628 (0.477-0.799)	0.762 (0.562-1.02)	0.862 (0.625-1.18)	0.968 (0.684-1.38)	1.09 (0.730-1.60)	1.26 (0.818-1.92)	1.41 (0.891-2.19)
10-min	0.483 (0.371-0.608)	0.585 (0.448-0.737)	0.751 (0.574-0.951)	0.889 (0.676-1.13)	1.08 (0.797-1.44)	1.22 (0.885-1.67)	1.37 (0.969-1.96)	1.54 (1.03-2.26)	1.79 (1.16-2.72)	1.99 (1.26-3.11)
15-min	0.569 (0.436-0.715)	0.688 (0.528-0.867)	0.883 (0.674-1.12)	1.05 (0.795-1.33)	1.27 (0.937-1.70)	1.44 (1.04-1.97)	1.61 (1.14-2.31)	1.81 (1.22-2.66)	2.11 (1.36-3.21)	2.35 (1.49-3.65)
30-min	0.790 (0.606-0.993)	0.954 (0.731-1.20)	1.22 (0.934-1.55)	1.45 (1.10-1.84)	1.75 (1.29-2.34)	1.98 (1.44-2.72)	2.23 (1.57-3.18)	2.50 (1.68-3.67)	2.91 (1.88-4.43)	3.24 (2.05-5.05)
60-min	1.01 (0.775-1.27)	1.22 (0.935-1.54)	1.56 (1.19-1.97)	1.85 (1.40-2.35)	2.24 (1.65-2.99)	2.53 (1.83-3.47)	2.84 (2.01-4.06)	3.19 (2.14-4.67)	3.71 (2.40-5.64)	4.13 (2.62-6.44)
2-hr	1.33 (1.03-1.66)	1.59 (1.23-1.99)	2.01 (1.55-2.53)	2.37 (1.81-2.99)	2.85 (2.12-3.79)	3.21 (2.34-4.37)	3.60 (2.56-5.11)	4.04 (2.72-5.87)	4.68 (3.04-7.08)	5.22 (3.31-8.07)
3-hr	1.55 (1.20-1.93)	1.84 (1.43-2.30)	2.33 (1.80-2.91)	2.73 (2.10-3.44)	3.29 (2.45-4.35)	3.70 (2.71-5.02)	4.14 (2.95-5.87)	4.65 (3.14-6.74)	5.40 (3.51-8.13)	6.02 (3.83-9.28)
6-hr	1.97 (1.54-2.43)	2.35 (1.83-2.91)	2.98 (2.32-3.70)	3.50 (2.70-4.37)	4.21 (3.16-5.55)	4.75 (3.50-6.41)	5.32 (3.82-7.50)	5.99 (4.06-8.62)	6.98 (4.55-10.4)	7.81 (4.98-12.0)
12-hr	2.43 (1.91-2.98)	2.93 (2.30-3.61)	3.76 (2.94-4.64)	4.44 (3.46-5.52)	5.38 (4.07-7.05)	6.08 (4.51-8.17)	6.83 (4.95-9.60)	7.73 (5.26-11.1)	9.09 (5.95-13.5)	10.2 (6.55-15.6)
24-hr	2.85 (2.26-3.48)	3.50 (2.77-4.27)	4.56 (3.59-5.58)	5.44 (4.26-6.71)	6.65 (5.06-8.67)	7.54 (5.64-10.1)	8.51 (6.22-12.0)	9.71 (6.62-13.8)	11.6 (7.59-17.1)	13.2 (8.45-19.9)
2-day	3.21 (2.56-3.89)	4.01 (3.19-4.86)	5.31 (4.21-6.46)	6.39 (5.04-7.82)	7.87 (6.05-10.2)	8.96 (6.76-12.0)	10.2 (7.51-14.3)	11.7 (8.01-16.6)	14.2 (9.33-20.8)	16.3 (10.5-24.5)
3-day	3.49 (2.79-4.20)	4.36 (3.49-5.27)	5.80 (4.62-7.03)	6.99 (5.54-8.52)	8.62 (6.65-11.2)	9.82 (7.44-13.1)	11.1 (8.27-15.6)	12.9 (8.82-18.1)	15.6 (10.3-22.9)	18.1 (11.6-27.0)
4-day	3.74 (3.00-4.50)	4.67 (3.75-5.63)	6.20 (4.95-7.49)	7.46 (5.93-9.08)	9.20 (7.11-11.9)	10.5 (7.96-13.9)	11.9 (8.84-16.6)	13.7 (9.42-19.3)	16.6 (11.0-24.3)	19.2 (12.4-28.6)
7-day	4.46 (3.60-5.33)	5.49 (4.43-6.58)	7.18 (5.78-8.63)	8.58 (6.86-10.4)	10.5 (8.16-13.5)	11.9 (9.09-15.7)	13.5 (10.0-18.7)	15.5 (10.7-21.6)	18.6 (12.3-26.9)	21.3 (13.8-31.6)
10-day	5.18 (4.20-6.17)	6.27 (5.08-7.48)	8.05 (6.49-9.63)	9.52 (7.64-11.5)	11.6 (8.99-14.7)	13.1 (9.95-17.1)	14.7 (10.9-20.2)	16.7 (11.6-23.2)	19.8 (13.2-28.7)	22.5 (14.6-33.3)
20-day	7.40 (6.04-8.75)	8.57 (6.99-10.2)	10.5 (8.52-12.5)	12.1 (9.75-14.4)	14.3 (11.1-17.9)	15.9 (12.1-20.5)	17.6 (13.0-23.7)	19.6 (13.7-27.0)	22.5 (15.0-32.2)	24.9 (16.2-36.5)
30-day	9.26 (7.60-10.9)	10.5 (8.58-12.3)	12.4 (10.2-14.7)	14.1 (11.4-16.8)	16.3 (12.8-20.4)	18.1 (13.8-23.0)	19.8 (14.6-26.3)	21.7 (15.2-29.8)	24.4 (16.3-34.7)	26.5 (17.2-38.6)
45-day	11.6 (9.53-13.6)	12.8 (10.5-15.1)	14.9 (12.2-17.5)	16.5 (13.5-19.6)	18.9 (14.8-23.3)	20.7 (15.8-26.1)	22.5 (16.5-29.4)	24.3 (17.0-33.1)	26.6 (17.9-37.7)	28.4 (18.5-41.3)
60-day	13.5 (11.1-15.8)	14.8 (12.2-17.3)	16.9 (13.9-19.8)	18.6 (15.2-22.0)	21.0 (16.5-25.8)	22.8 (17.5-28.7)	24.7 (18.1-32.0)	26.4 (18.6-35.8)	28.5 (19.2-40.3)	30.0 (19.6-43.5)

[1] Precipitation frequency (PF) estimates in this table are based on frequency analysis of partial duration series (PDS).

Numbers in parenthesis are PF estimates at lower and upper bounds of the 90% confidence interval. The probability that precipitation frequency estimates (for a given duration and average recurrence interval) will be greater than the upper bound (or less than the lower bound) is 5%. Estimates at upper bounds are not checked against probable maximum precipitation (PMP) estimates and may be higher than currently valid PMP values.

Please refer to NOAA Atlas 14 document for more information.

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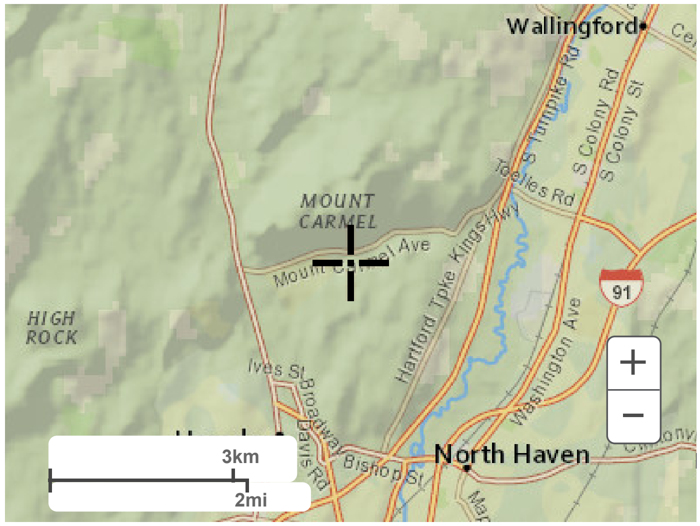
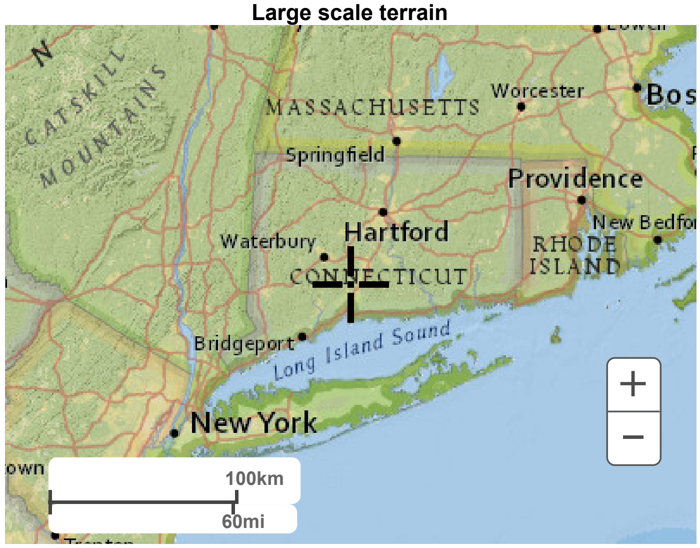
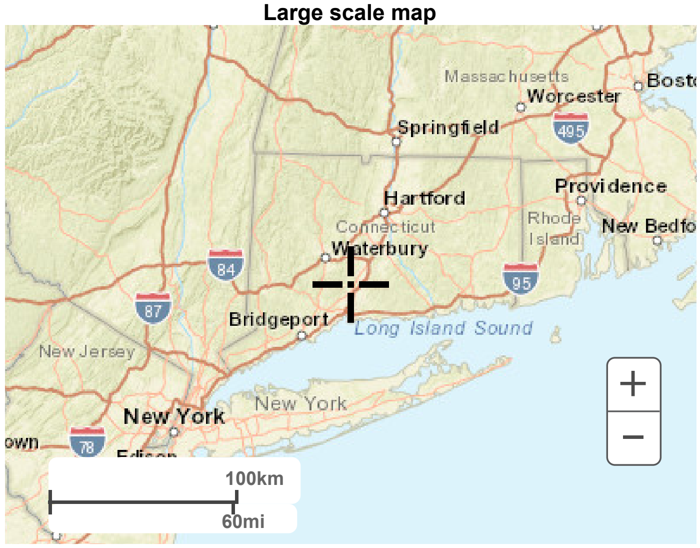
PF graphical



Large scale terrain



Large scale map



Large scale aerial

ATTACHMENT G

HYDROLOGIC ANALYSIS – COMPUTER MODEL RESULTS

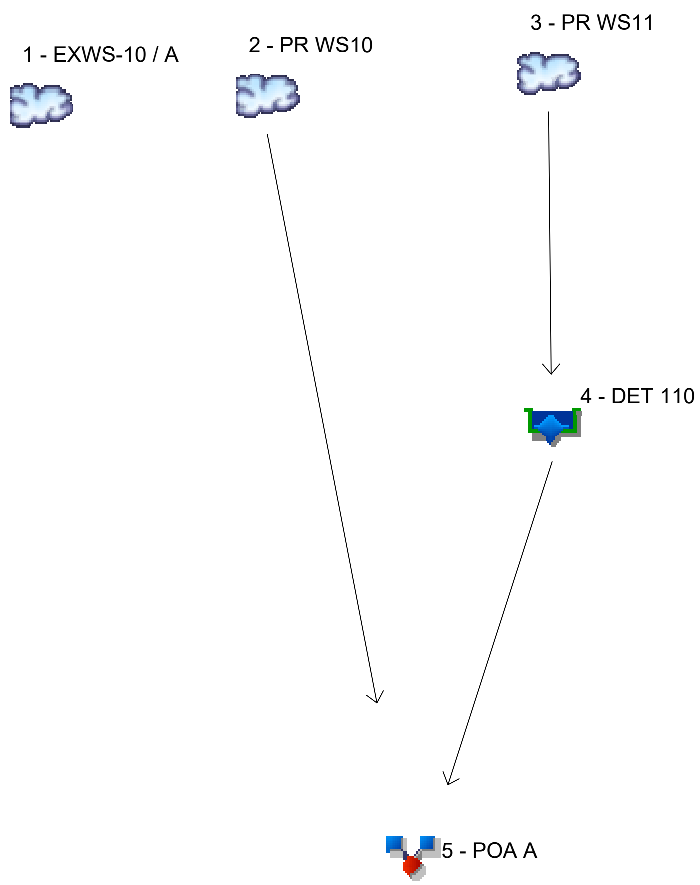
Hydrographs Peak Flowrate Summary (cfs)
Existing vs. Proposed

Storm Event	**2yr**		**10yr**		**25yr**		**50yr**		**100yr**	
	Exist	**Prop**	**Exist**	**Prop**	**Exist**	**Prop**	**Exist**	**Prop**	**Exist**	**Prop**
Point of Analysis A	4.6	4.0	11.1	10.4	15.6	14.7	18.9	18.2	22.7	22.7
DET 110 W.S. Elev. (ft.) Top of Berm Elev. = 159.0	--	156.90	--	157.43	--	157.67	--	157.82	--	157.92

Study Area	**Description**
A	Wetland System - West

Watershed Model Schematic

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020



Legend

Hyd.	Origin	Description
1	SCS Runoff	EXWS-10 / A
2	SCS Runoff	PR WS10
3	SCS Runoff	PR WS11
4	Reservoir	DET 110
5	Combine	POA A

Project: SU-Model01.gpw

Sunday, 10 / 25 / 2020

Hydraflow Table of Contents

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Sunday, 10 / 25 / 2020

Hydrograph Return Period Recap

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Inflow hyd(s)	Peak Outflow (cfs)								Hydrograph Description
			1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr	
1	SCS Runoff	------	-------	4.626	-------	-------	11.13	15.57	18.94	22.67	EXWS-10 / A
2	SCS Runoff	------	-------	1.894	-------	-------	4.684	6.606	8.068	9.689	PR WS10
3	SCS Runoff	------	-------	4.032	-------	-------	8.135	10.79	12.75	14.89	PR WS11
4	Reservoir	3	-------	2.533	-------	-------	6.326	8.800	11.05	13.46	DET 110
5	Combine	2, 4	-------	4.019	-------	-------	10.39	14.68	18.21	22.71	POA A

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	4.626	3	729	0.446	------	------	------	EXWS-10 / A
2	SCS Runoff	1.894	3	729	0.185	------	------	------	PR WS10
3	SCS Runoff	4.032	3	729	0.367	------	------	------	PR WS11
4	Reservoir	2.533	3	741	0.363	3	156.90	0.097	DET 110
5	Combine	4.019	3	738	0.549	2, 4	------	------	POA A

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	11.13	3	729	1.013	------	------	------	EXWS-10 / A
2	SCS Runoff	4.684	3	729	0.427	------	------	------	PR WS10
3	SCS Runoff	8.135	3	729	0.735	------	------	------	PR WS11
4	Reservoir	6.326	3	738	0.732	3	157.43	0.147	DET 110
5	Combine	10.39	3	735	1.159	2, 4	------	------	POA A

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	15.57	3	729	1.407	------	------	------	EXWS-10 / A
2	SCS Runoff	6.606	3	729	0.598	------	------	------	PR WS10
3	SCS Runoff	10.79	3	729	0.980	------	------	------	PR WS11
4	Reservoir	8.800	3	735	0.977	3	157.67	0.173	DET 110
5	Combine	14.68	3	732	1.574	2, 4	------	------	POA A

SU-Model01.gpw	Return Period: 25 Year	Sunday, 10 / 25 / 2020

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	18.94	3	729	1.710	------	------	------	EXWS-10 / A
2	SCS Runoff	8.068	3	729	0.728	------	------	------	PR WS10
3	SCS Runoff	12.75	3	729	1.165	------	------	------	PR WS11
4	Reservoir	11.05	3	735	1.161	3	157.82	0.188	DET 110
5	Combine	18.21	3	732	1.889	2, 4	------	------	POA A

SU-Model01.gpw	Return Period: 50 Year	Sunday, 10 / 25 / 2020

Hydrograph Summary Report

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® 2019 by Autodesk, Inc. v2020

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	22.67	3	729	2.049	------	------	------	EXWS-10 / A
2	SCS Runoff	9.689	3	729	0.875	------	------	------	PR WS10
3	SCS Runoff	14.89	3	729	1.368	------	------	------	PR WS11
4	Reservoir	13.46	3	732	1.365	3	157.92	0.198	DET 110
5	Combine	22.71	3	732	2.240	2, 4	------	------	POA A

Pond Report

Pond No. 1 - DET 110

Pond Data

Contours -User-defined contour areas. Conic method used for volume calculation. Begining Elevation = 155.70 ft

Stage / Storage Table

Stage (ft)	Elevation (ft)	Contour area (sqft)	Incr. Storage (acft)	Total storage (acft)
0.00	155.70	3,025	0.000	0.000
0.30	156.00	3,275	0.022	0.022
0.80	156.50	3,675	0.040	0.062
1.30	157.00	4,050	0.044	0.106
1.80	157.50	4,450	0.049	0.155
2.30	158.00	4,875	0.053	0.208
2.80	158.50	5,650	0.060	0.268
3.30	159.00	6,425	0.069	0.338

Culvert / Orifice Structures

	[A]	[B]	[C]	[PrfRsr]
Rise (in)	= 18.00	0.00	0.00	0.00
Span (in)	= 18.00	0.00	0.00	0.00
No. Barrels	= 1	0	0	0
Invert El. (ft)	= 153.00	0.00	0.00	0.00
Length (ft)	= 34.00	0.00	0.00	0.00
Slope (%)	= 1.47	0.00	0.00	n/a
N-Value	= .012	.013	.013	n/a
Orifice Coeff.	= 0.60	0.60	0.60	0.60
Multi-Stage	= n/a	No	No	No

Weir Structures

	[A]	[B]	[C]	[D]
Crest Len (ft)	= 11.40	0.00	0.00	0.00
Crest El. (ft)	= 157.75	155.70	0.00	0.00
Weir Coeff.	= 3.33	1.62	3.33	3.33
Weir Type	= 1	65 degV	---	---
Multi-Stage	= Yes	Yes	No	No

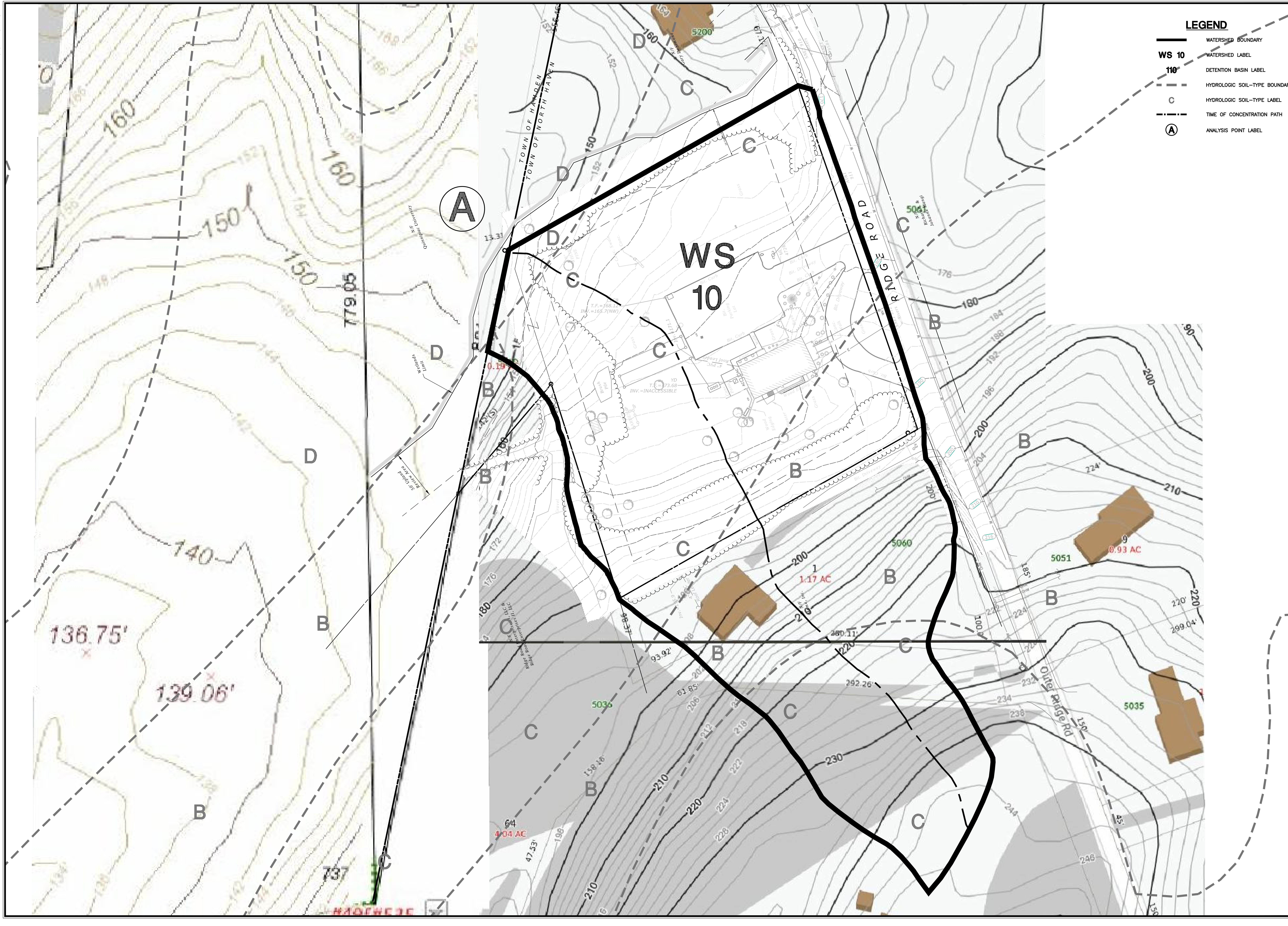
Exfil.(in/hr) = 0.000 (by Wet area)
TW Elev. (ft) = 0.00

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

Stage / Storage / Discharge Table

Stage ft	Storage acft	Elevation ft	Clv A cfs	Clv B cfs	Clv C cfs	PrfRsr cfs	Wr A cfs	Wr B cfs	Wr C cfs	Wr D cfs	Exfil cfs	User cfs	Total cfs
0.00	0.000	155.70	0.00	---	---	---	0.00	---	---	---	---	---	0.000
0.30	0.022	156.00	11.88 ic	---	---	---	0.00	0.08	---	---	---	---	0.080
0.80	0.062	156.50	11.88 ic	---	---	---	0.00	0.93	---	---	---	---	0.926
1.30	0.106	157.00	11.88 ic	---	---	---	0.00	3.12	---	---	---	---	3.118
1.80	0.155	157.50	11.88 ic	---	---	---	0.00	7.03	---	---	---	---	7.033
2.30	0.208	158.00	15.30 ic	---	---	---	4.75	10.55 s	---	---	---	---	15.30
2.80	0.268	158.50	18.38 ic	---	---	---	12.07 s	6.31 s	---	---	---	---	18.38
3.30	0.338	159.00	19.45 ic	---	---	---	13.73 s	5.71 s	---	---	---	---	19.44

ATTACHMENT H
WATERSHED MAPS



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W E
S
0' 20' 40'
0 1/2" 1"

MILONE & MACBROOM
NOW PART OF SLR
99 REALTY DRIVE
CHESHIRE, CT 06410
203.271.1773
WWW.MMINC.COM

DESCRIPTION	DATE	BY

WATERSHED MAP - EXISTING CONDITIONS

SLATE UPPER SCHOOL

5100 RIDGE ROAD
NORTH HAVEN, CONNECTICUT

DESIGNED	DRAWN	CHECKED
FAB/MCB	MCB	FAB

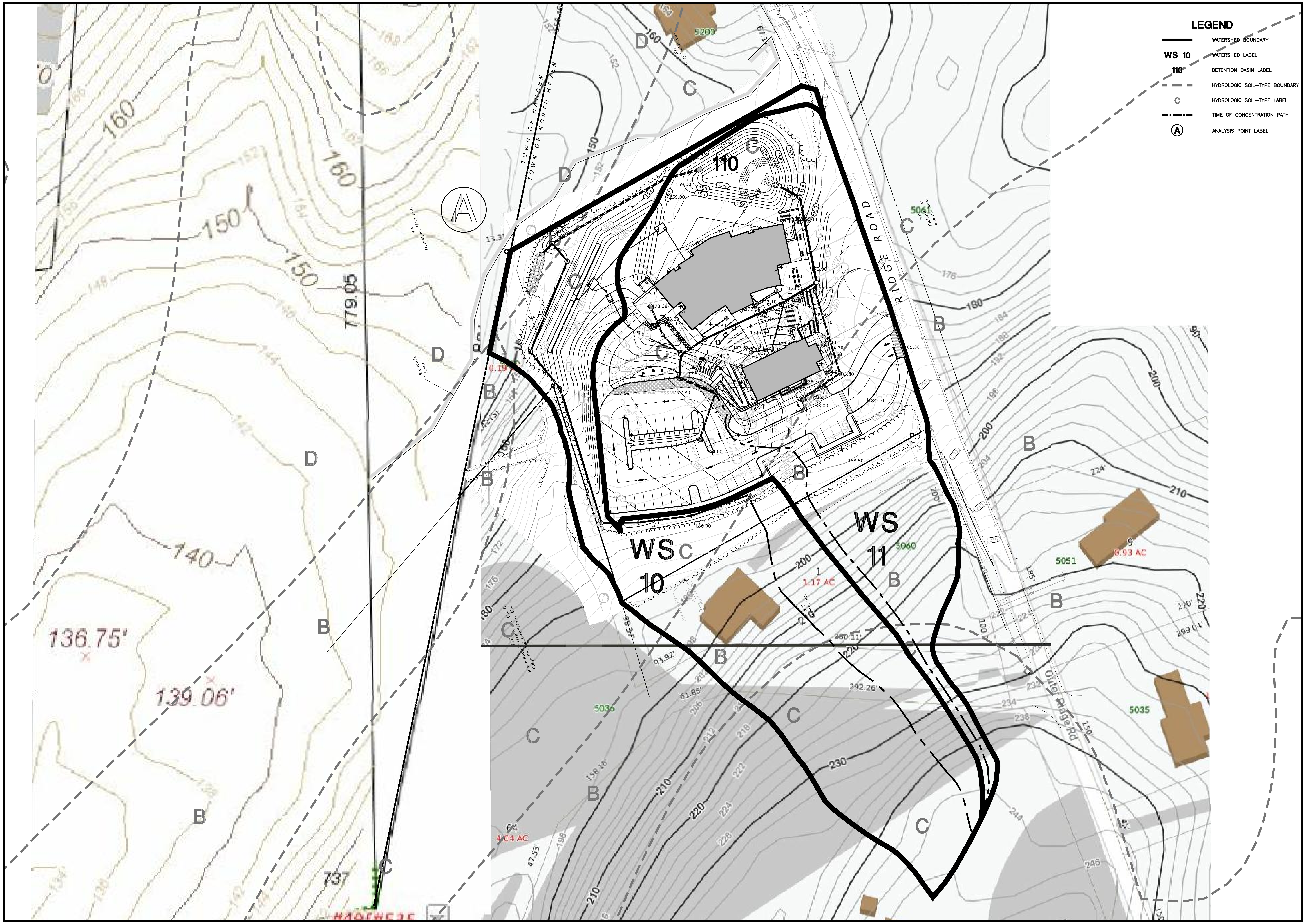
SCALE: 1"=40'

DATE: OCTOBER 27, 2020

PROJECT NO.: 6156-03

SHEET NO.: 1 OF 2

SHEET NAME: EXWS



MILONE & MACBROOM
NOW PART OF SLR
99 REALTY DRIVE
CHESHIRE, CT 06410
203.271.1773
WWW.MMINC.COM

DESCRIPTION	DATE	BY

WATERSHED MAP - PROPOSED CONDITIONS

SLATE UPPER SCHOOL

5100 RIDGE ROAD
NORTH HAVEN, CONNECTICUT

FAB/MCB	MCB	FAB
DESIGNED	DRAWN	CHECKED

SCALE: 1"=40'

DATE: OCTOBER 27, 2020

PROJECT NO.: 6156-03

SHEET NO.: 2 OF 2

SHEET NAME: PRWS